

## MEMORANDUM

**TO:** The Honorable Mayor and City Council  
**FROM:** Joseph Tebrugge, Director of Engineering  
**DATE:** February 27, 2023  
**SUBJECT:** East Loop Road Corridor Traffic Study

East Loop Road is an important Major Collector Road that provides connection between Naperville Road and Butterfield Road in southeast Wheaton. The corridor was last studied in 1994 and a study was required to ensure that cumulative traffic effects on the roadway would not cause negative impacts on City of Wheaton residents. The East Loop Road Corridor Traffic Study was required as per Ordinance O-2022-44, (attached as Exhibit A), which granted a special use permit for the planned unit development of a WashU car wash in the PUD on the northeast corner of East Loop Road and Butterfield Road.

The traffic study (attached as Exhibit B) was completed by Kenig, Lindgren, O'Hara, Aboona, Inc (KLOA). It analyzed existing traffic conditions, future 2028 traffic conditions with the car wash, and future 2028 traffic conditions with the car wash and a possible additional restaurant development on the north end of the car wash parking lot. All 2028 future conditions assume that the adjoining properties such as the Rice Lake Square shopping center and IIT are fully operating and no longer have any vacancies.

The East Loop Road Corridor Traffic Study also provided insight on the following possible changes to the corridor: 1) Adding a turn lane at East Loop Road and Butterfield, 2) Adding stop control or a traffic signal at East Loop Road and the Chic-Fil-A access drive, 3) Relocating the Chic-Fil-a access drive and/or adding an access drive, and 4) Attempting to alter the right turn traffic coming onto East Loop Road from Butterfield.

The results of the study illustrate that the corridor is designed well and works very well and as expected for a busy major collector road. No traffic signs or signals are warranted and access drives should not be moved or added.

The future 2028 condition shows that the East Loop Road Corridor has adequate reserve capacity to handle the increase in traffic even with all the adjoining properties fully operating. In fact, all of the intersections and turning movements continue to work very well with the



exception of the left hand turns out of the Chic-Fil-A access drive which start to experience an increased delay.

Should a potential future high traffic use development occur on the north part of the PUD containing the Chic-Fil-A/WashU businesses, the traffic flow on East Loop Road will remain unchanged. However, the traffic exiting the access drive out of the property would then break down into a very low level of service and would experience much longer queue times. To ensure that this does not occur, the owner of the WashU development has suggested to the City that the north parking lot can be deed restricted to only allow a use with low traffic volumes. This would not allow an establishment with high traffic volumes, such as a quick service restaurant, to be built.

Currently Ordinance O-2022-44 requires that some of the existing trees located at the southeast corner of East Loop Road and the Access Drive be removed and replaced with lower shrubs to improve the safety and visibility of this intersection. Should City Council desire any additional improvements, such as a deed restriction, the previous ordinance would have to be amended.

# EXHIBIT A

## ORDINANCE NO. O-2022-44

**AN ORDINANCE AMENDING CITY OF WHEATON ORDINANCE NOS. E-3972, F-1471, F-1480, AND O-2021-09 "AN ORDINANCE CONDITIONALLY AMENDING THE WHEATON ZONING MAP AND CONDITIONALLY GRANTING A SPECIAL USE PERMIT FOR PLANNED UNIT DEVELOPMENT ON A CERTAIN PIECE OF PROPERTY COMMONLY LOCATED AT THE NORTHEAST CORNER OF EAST LOOP ROAD AND BUTTERFIELD GRADY'S/ COZYMEL'S RESTAURANTS"**  
**(WASHU REAL ESTATE, LLC.)**

**WHEREAS**, on March 21, 1994, the City of Wheaton, Illinois, ("City"), enacted City Ordinance No. E-3972 ("Original Ordinance"), which granted a special use permit for a planned unit development to allow the construction and use of two (2) restaurants, all on property commonly known as 301 and 311 East Loop Road, Wheaton, Illinois 60189; and

**WHEREAS**, on February 1, 2010, the City enacted City Ordinance No. F-1471 ("First Amended Ordinance"), which amended the special use permit for a planned unit development to allow the demolition of the existing restaurant on the property commonly known as 301 East Loop Road in order to construct a new restaurant with a drive-thru lane; and

**WHEREAS**, on April 19, 2010, the City enacted City Ordinance No. F-1480 ("Second Amended Ordinance"), which further amended the special use permit for a planned unit development to revise the drive-thru lane configuration (to allow a multi-point ordering layout) on property commonly 301 East Loop Road; and

**WHEREAS**, on March 1, 2021, the City enacted City Ordinance No. O-2021-09 ("Third Amended Ordinance"), which further amended the special use permit for a planned unit development to allow the construction of a dual lane drive-thru configuration (which necessitated the construction of a small addition on the rear of the restaurant, face-to-face order, delivery canopies and several other site plan modifications) on property commonly 301 East Loop Road; and

**WHEREAS**, an application has been submitted to further amend the special use permit for a planned unit development approved in the: Original Ordinance; First Amended Ordinance; Second Amended Ordinance; and Third Amended Ordinance to allow the demolition of the existing vacant restaurant on the property commonly known as 311 East Loop Road in order to construct a car wash; and

**WHEREAS**, pursuant to notice as required by the Illinois Municipal Code and the Wheaton Zoning Ordinance, a public hearing was conducted by the Wheaton Planning and Zoning Board on July 26, 2022, August 9, 2022, and August 23, 2022, to consider the issuance of an amendment to the special use permit for a planned unit development; and the Board has recommended the issuance of the special use permit for a planned unit development; and

**WHEREAS**, the City Council has reviewed the public hearing record before the Wheaton Planning and Zoning Board, as well as the exhibits for the proposed car wash, and considered a staff recommendation, and has concluded in its legislative judgment that the information and testimony presented at the public hearing was inadequate to properly determine what, if any, traffic congestion minimizing efforts may be necessary, or prudent, as conditions to be imposed on the special use permit for this specific car wash in a manner consistent with the public safety, health and welfare; and

**WHEREAS**, the law provides that a special use permit, allowed in a zoning ordinance for a business, is tantamount to a permitted use, subject to imposition of lawful conditions. *Paul v. County of Ogle*, 218 Ill. App. 2d 170696, 422 Ill. Dec. 453 (2<sup>nd</sup> Dist. 2018); and

**WHEREAS**, for the City Council to determine what lawful special use permit conditions for the proposed car wash are necessary or prudent, the City Council has concluded a more detailed traffic study and report than what has been submitted to date is necessary.

**NOW, THEREFORE BE IT ORDAINED** by the City Council of the City of Wheaton, DuPage County, Illinois, pursuant to its home rule powers, as follows:

**Section 1:** The following described property has been, and continues to be, zoned and classified in the C-5 Planned Commercial District zoning classification:

LOT 2 IN DANADA FARMS EAST UNIT 4, BEING A SUBDIVISION OF LOT 11 IN DANADA FARMS EAST UNIT 1, BEING A SUBDIVISION OF PART OF SECTION 28, AND THE NORTH 1/2 OF SECTION 33, IN TOWNSHIP 39 NORTH, RANGE 10 EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO THE PLAT THEREOF RECORDED APRIL 10, 1996 AS DOCUMENT R96-058160, IN DUPAGE COUNTY, ILLINOIS.

TOGETHER WITH THOSE CERTAIN NON-EXCLUSIVE EASEMENTS AS GRANTED AND MORE FULLY SET FORTH IN MUTUAL EASEMENT AND USE AGREEMENT DATED DECEMBER 21, 1995 AND RECORDED JUNE 21, 1996 AS DOCUMENT R96-103888, MADE BY AND BETWEEN PACIFIC INDUSTRIAL PROPERTIES HOLDINGS, INC. AND GRADY'S AMERICAN GRILL RESTAURANT CORPORATION.

PIN: 05-28-403-029

**Section 2:** An amendment to the Original Ordinance, First Amended Ordinance, Second Amended Ordinance, and Third Amended Ordinance to allow the demolition of the existing restaurant on the property commonly known as 311 East Loop Road in order to construct a new car wash is granted in full compliance with the following plans: "WashU Wheaton – Preliminary Engineering Plans," prepared by Merit Corp, Aurora, IL, sheets 1-11, dated June 30, 2022; "WashU Wheaton – Bold Curb Exhibit", prepared by Merit Corp, Aurora, IL, sheet 1-1, dated August 11, 2022; "Peak Times Proposed Traffic Flow Patterns", prepared by KLOA, Rosemont, IL, sheet 1-1, dated August 21, 2022, "WashU Car Wash", prepared by Design Perspectives, Naperville, IL, sheets TP 100, LP 101, 102, 103, 500, 501, 502, and 503, dated July 1, 2022, "WashU Elevations and Floor Plan", prepared by WashU, sheets 1-2; and "WashU Sign Package, prepared by Midwest Sign Company, Wayland, MI, sheets 1-4, dated May 16, 2022, subject to the following conditions:

1. Additional shrubs shall be added in the landscaping area between Chick-fil-A and the car wash.
2. Some of the existing trees located at the southeast corner of East Loop and the access drive shall be removed (and replaced with lower shrubs) to improve safety and visibility as depicted on the revised land swap exhibit, subject to approval by the Director of Planning of Economic Development.
3. The drive lanes shall be reconfigured to improve outbound queuing as depicted on the revised land swap exhibit, subject to approval by the Director of Engineering.
4. Even though the northern portion of the property is not being redeveloped at this time, the parking lot lights, and the light remnant shall be replaced with new lights to match the parking lot lights for the car wash.

5. A mainline water valve shall be installed by the applicant east of the proposed water connection to improve the City of Wheaton system and the costs of the material and installation shall be paid for by the applicant; the City shall maintain the installed mainline water valve.
6. The preliminary engineering plan shall be subject to further staff review prior to the issuance of a site development permit.
7. An independent traffic study and report, contracted by the City, evaluating and recommending possible, necessary, or prudent improvements, to protect the public health, safety, and welfare by minimizing ingress and egress traffic congestion at the property's current intersection with East Loop Road, shall be completed prior to the final determination of what conditions should be imposed on the special use permit. The cost of this independent traffic report shall be divided equally between the applicant and the City.
8. A precondition to approval of the special use permit shall also include the applicant's agreement to pay one hundred percent (100%) of the recommended traffic improvements exclusive to the property proposed by the independent traffic report and a proportionate share of the costs not exclusive to the property recommended by the independent traffic report, that are necessary or prudent to minimize the negative traffic impacts.
9. The Corporate Authorities shall determine whether said improvements, or no improvements, recommended by the independent traffic study to minimize traffic congestion satisfy the standards for issuance of a special use permit as set forth in Section 5.10D of the Wheaton Zoning Ordinance.

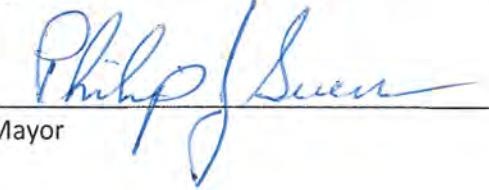
Section 3: In all other respects, the terms and provisions of the Original Ordinance, First Amended Ordinance, Second Amended Ordinance, and Third Amended Ordinance are ratified and remain in full force and effect.

Section 4: All ordinances and parts of ordinances in conflict with these provisions are hereby repealed.

Section 5: This ordinance shall become effective after its passage, approval, and publication in pamphlet form in the manner prescribed by law. The special use permit shall not become effective until the conditions imposed by this ordinance are fully satisfied.

ATTEST:

  
Andrea Ronedale  
City Clerk

  
Philip J. Suess  
Mayor

Roll Call Vote

Ayes:	Councilman Weller Councilwoman Bray-Parker Councilman Barbier Councilman Brown Mayor Suess Councilwoman Robbins
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Nays: None

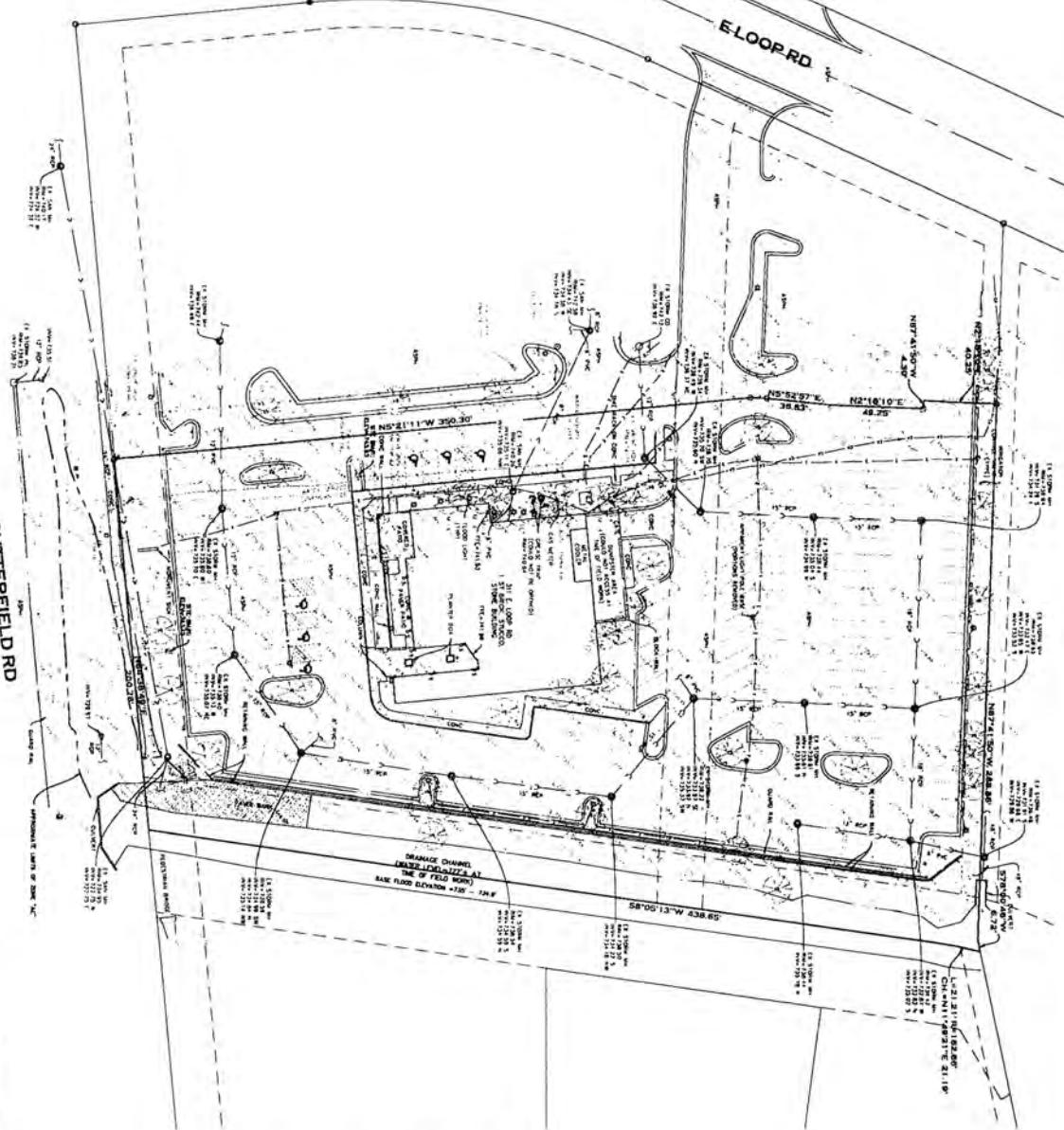
Absent: Councilwoman Fitch

Motion Carried Unanimously

Passed: October 17, 2022

Published: October 18, 2022





SURVEYOR NOTES

WASH-U WHEATON  
311 E. LOOP ROAD  
WHEATON, ILLINOIS 60189

## PRELIMINARY ENGINEERING PLANS

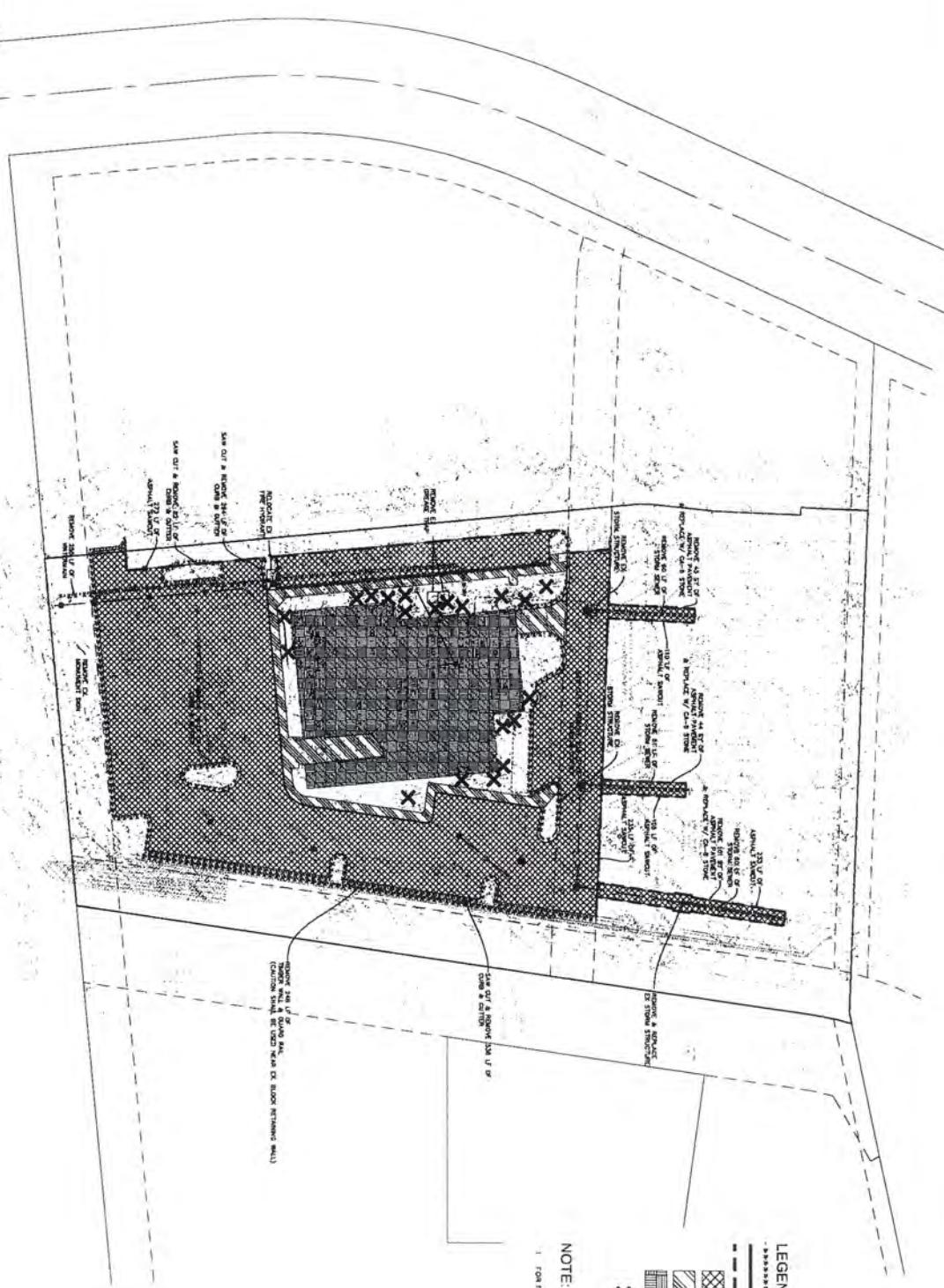
## EXISTING CONDITIONS PLAN



The logo for MeritCorp, featuring the word "MeritCorp" in a stylized, italicized font where the letters are interconnected.

DATE:	DESCRIPTION:
06/30/2022	ISSUED FOR REVIEW

MeritCorp

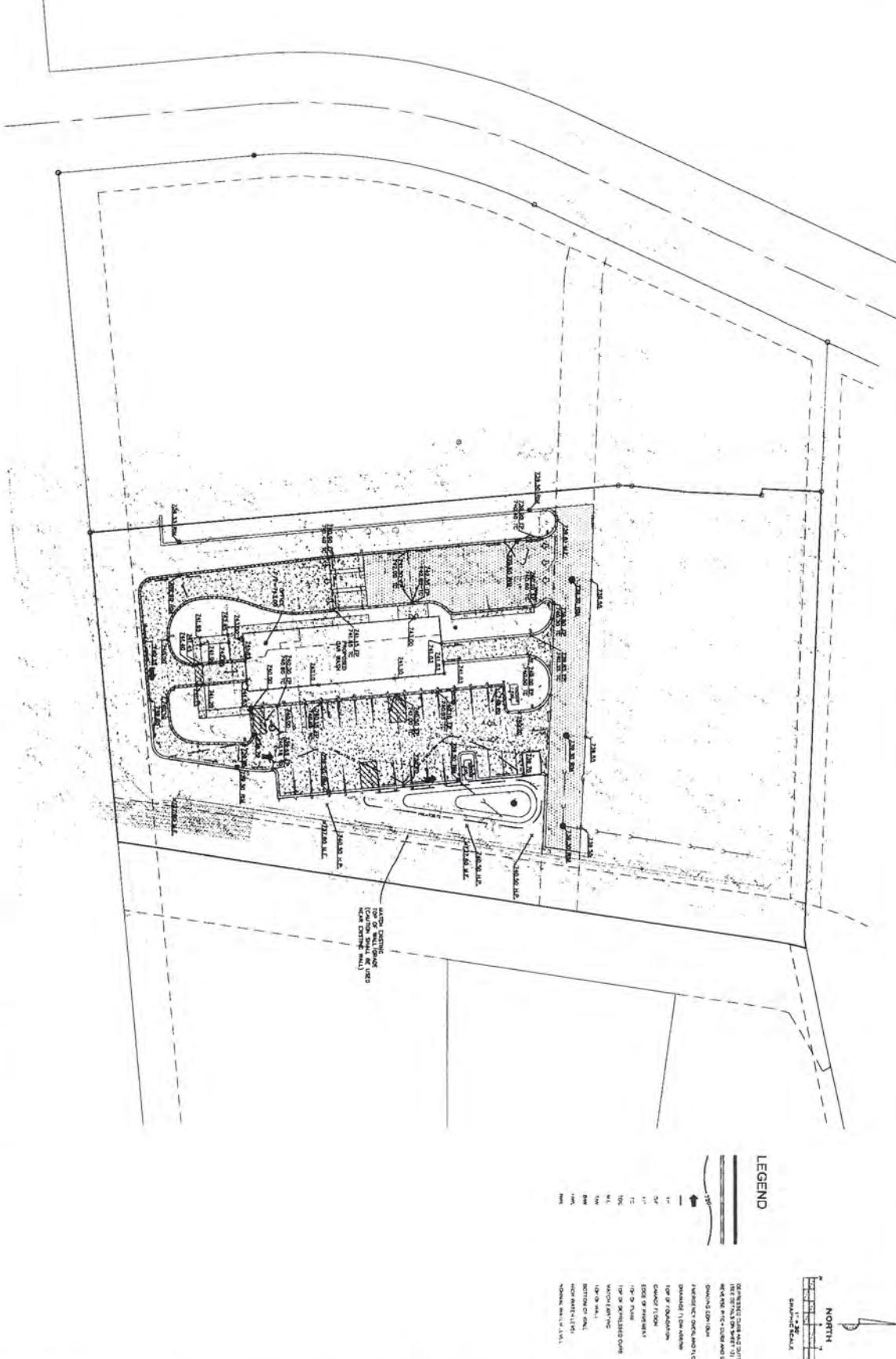


NOTE:

1. FOR FREE REMOVAL REFER TO LANDSCAPE PLAN

DATE: 06/30/2022 DESCRIPTION: ISSUED FOR REVIEW





WASH-U WHEATON  
311 E. LOOP ROAD  
WHEATON, ILLINOIS 60189

## PRELIMINARY ENGINEERING PLANS

PROJECT NO. M21180  
DRAWN BY: ZDS & JDS  
CHECKED BY: CLS  
SHEET NO. 5/11

11

104

4

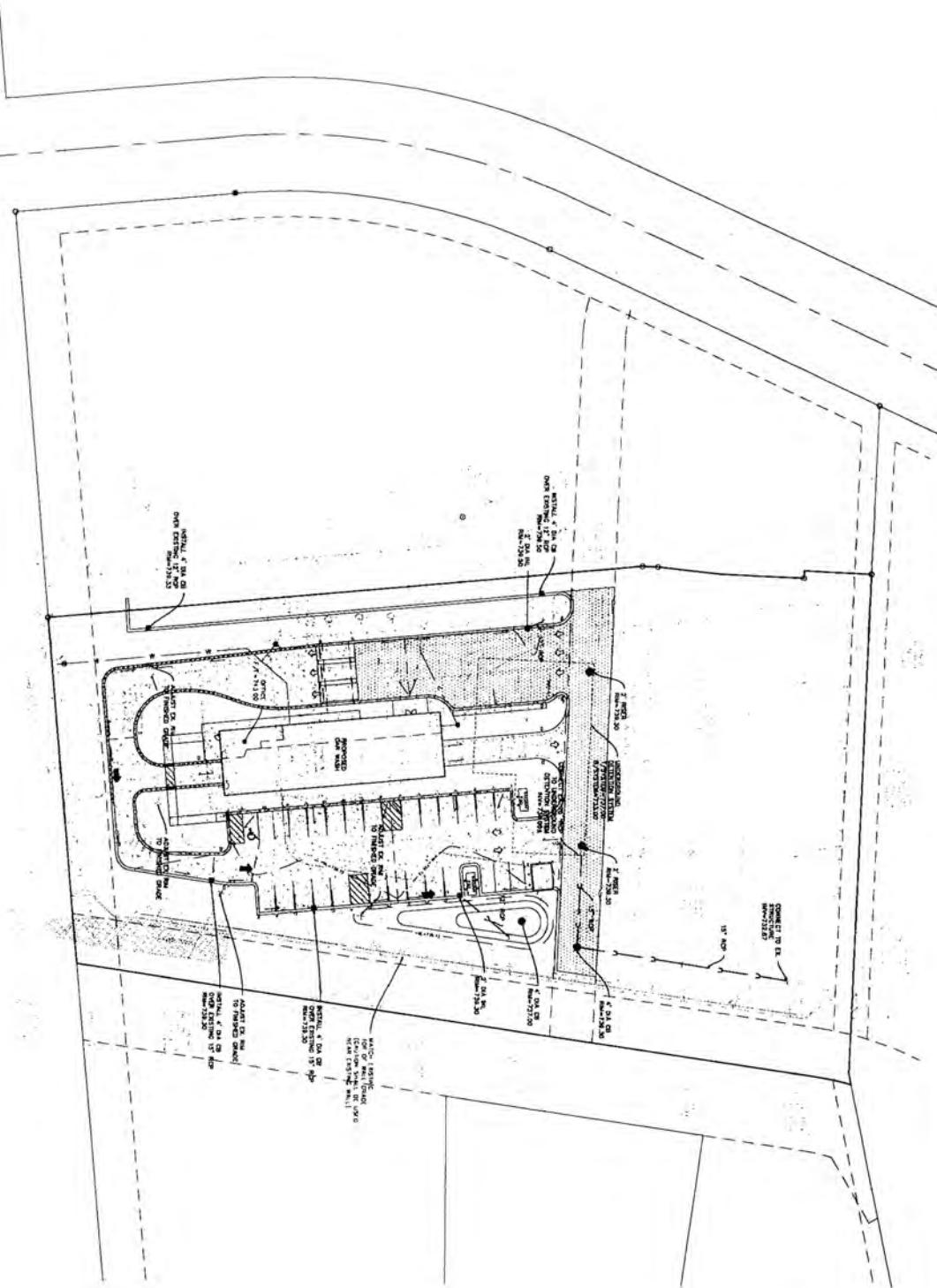
PR

The image shows two logos. On the left is the 'Wash-U Wheaton' logo, which includes the text 'WASH-U WHEATON' and '311 E. LOOP ROAD EATON, ILLINOIS 60189' above a circular emblem. The circular emblem features a stylized figure, possibly a Native American, in profile, facing left. On the right is the 'Merit' logo, which consists of the word 'Merit' in a large, bold, serif font next to a smaller circular emblem. The circular emblem contains a stylized figure, possibly a Native American, in profile, facing right. Below the 'Merit' logo is the address '4222 Meridian Parkway, Suite 112' and the phone number 'Aurora, IL 60504'.

**Other Office Locations:**

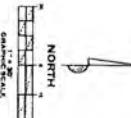
**DESCRIPTION:**  
**ISSUED FOR REVIEW**

MeritCorps  
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LEGEND

 Storm Sewer  
 Street  
 Water Main  
 Storm Drainage  
 Storm Catcher  
 Inlet



**WASH-U WHEATON**  
**311 E. LOOP ROAD**  
**WHEATON, ILLINOIS 60189**

PRELIMINARY ENGINEERING PLANS

STORM SEWER PLAN



**MeritCorp**  
 4222 Meridian Parkway, Suite 112  
 Aurora, IL 60504  
 Office 630.554.6655  
 Lic. No. 184.005680  
 Other Office Locations:  
[www.meritcorp.com](http://www.meritcorp.com) [Guinea, IL](#)

DATE: 06/30/2022 DESCRIPTION: ISSUED FOR REVIEW

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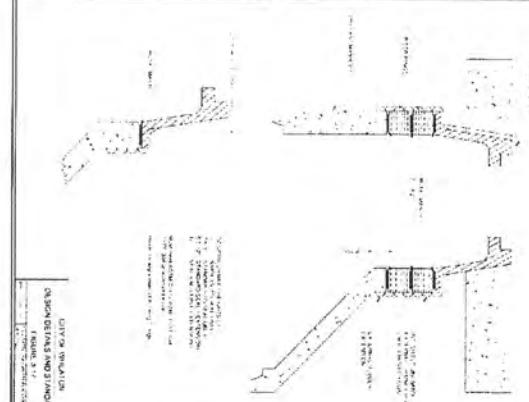
PROJECT NO. M211160  
 DRAWN BY: ZDS & JDS  
 CHECKED BY: CLS  
 SHEET NO. 6 / 1



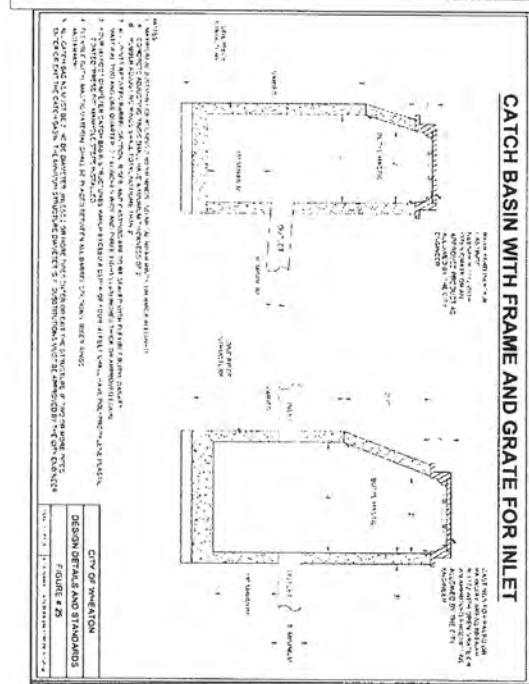


## SANITARY MANHOLE EXTERNAL CHIMNEY SEAL

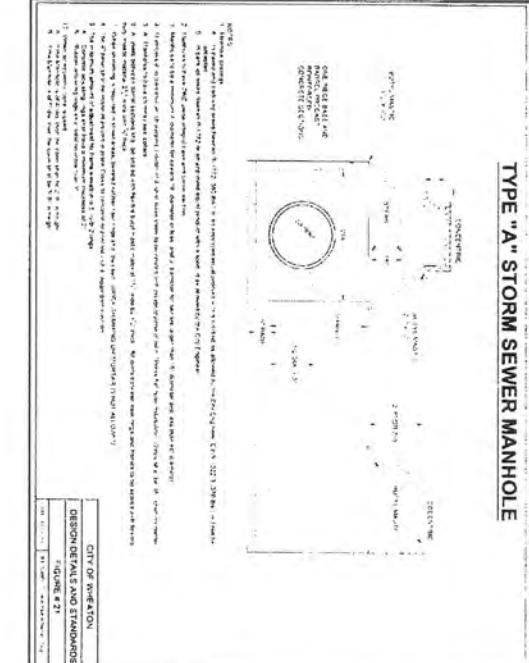
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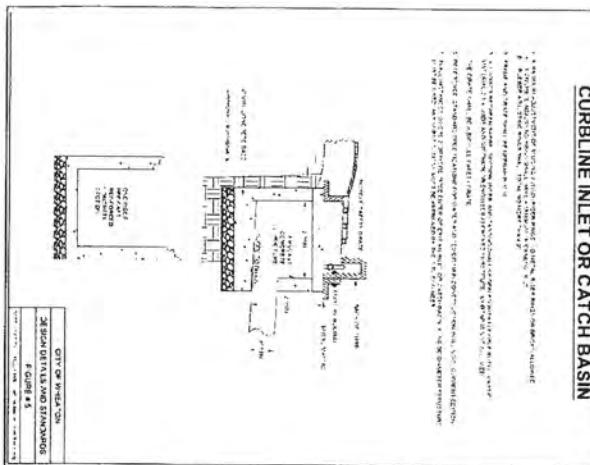
## CATCH BASIN WITH FRAME AND GRATE FOR INLET



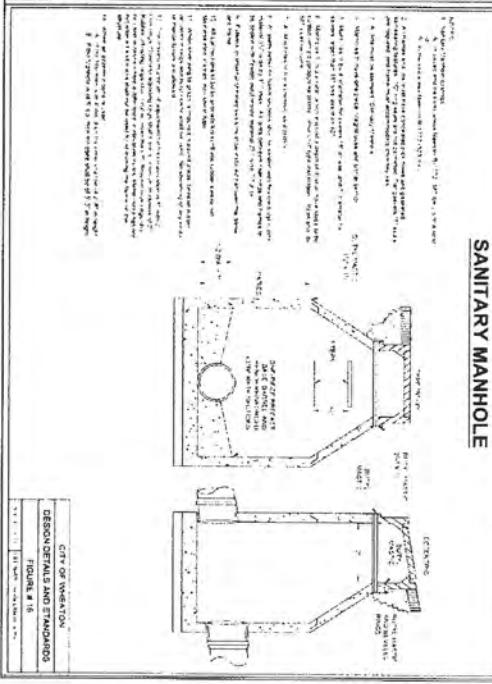
## TYPE "A" STORM SEWER MANHOLE



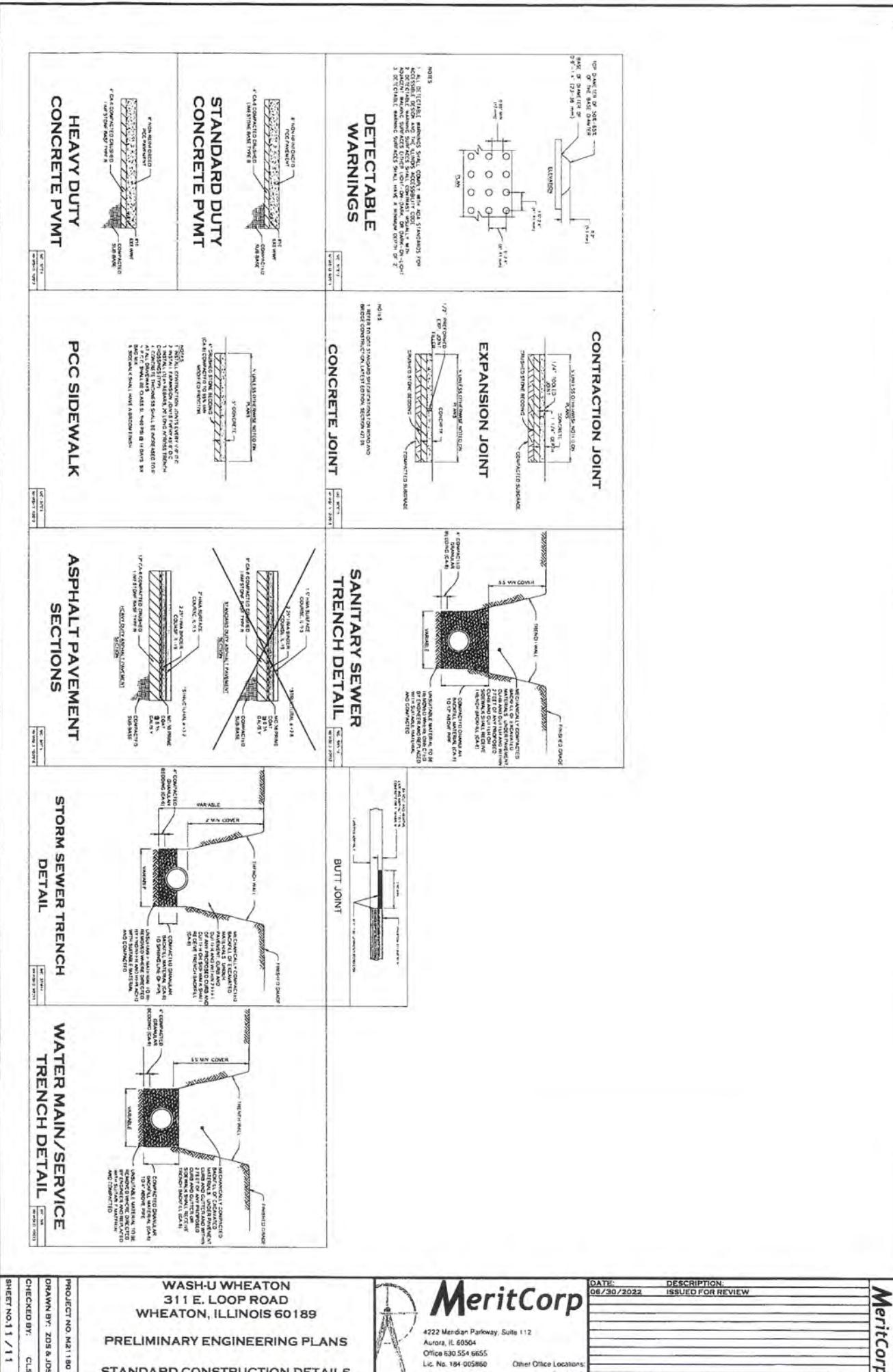
## CURBLINE INLET OR CATCH BASIN



## SANITARY MANHOLE







WASH-U WHEATON  
311 E. LOOP ROAD  
WHEATON, ILLINOIS 60189

**PRELIMINARY ENGINEERING PLANS**  
**STANDARD CONSTRUCTION DETAILS**

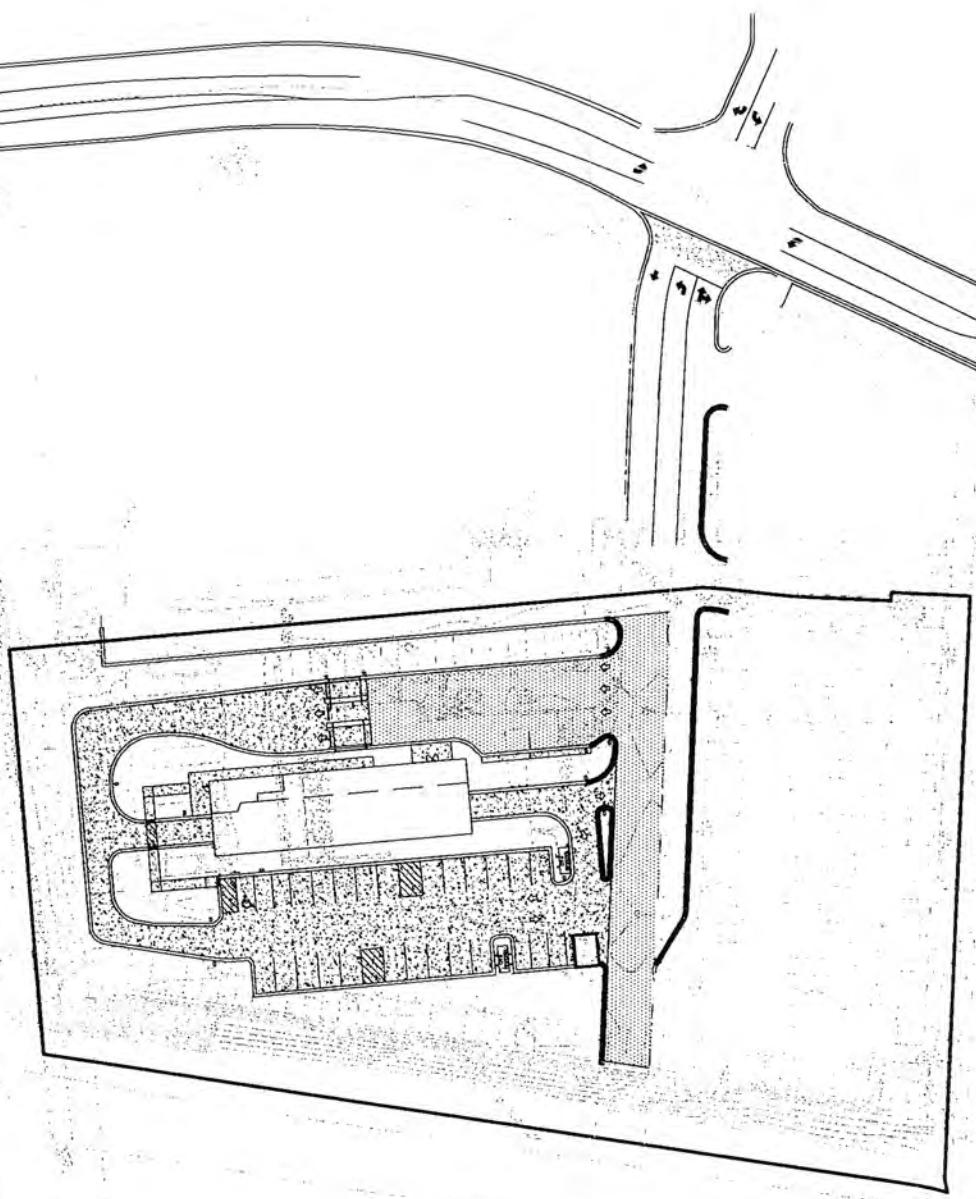
**MeritCorp**

4222 Meridian Parkway, Suite 112  
Aurora, IL 60504  
Office 630.554.6655  
Lic. No. 184-005860  
[www.meritcorp.com](http://www.meritcorp.com) Other

Other Office Locations:  
Gurnee, IL

DATE:	DESCRIPTION:
06/30/2022	ISSUED FOR REVIEW

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DATE: 08-11-2022

DESCRIPTION: ISSUED FOR REVIEW

WASH-UP WHEATON

311 E. LOOP ROAD  
WHEATON, ILLINOIS  
BOLD CURB EXHIBIT

PROJECT No.: M21180

DRAWN BY: ZDS

CHECKED BY: CLS

REVIEWED BY: CLS

APR 11 2022

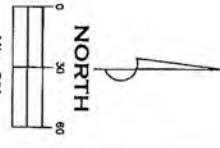
MERITCORP

www.meritcorp.com

GRAPHIC SCALE

1" = 60'

NORTH

Engineering - Planning - Surveying - Environmental  
4222 Mendota Parkway, Suite 112  
Aurora, IL 60504  
Office 630.554.6655  
Lic. No. 184-005860  
Other Office Locations:  
Gurnee, IL

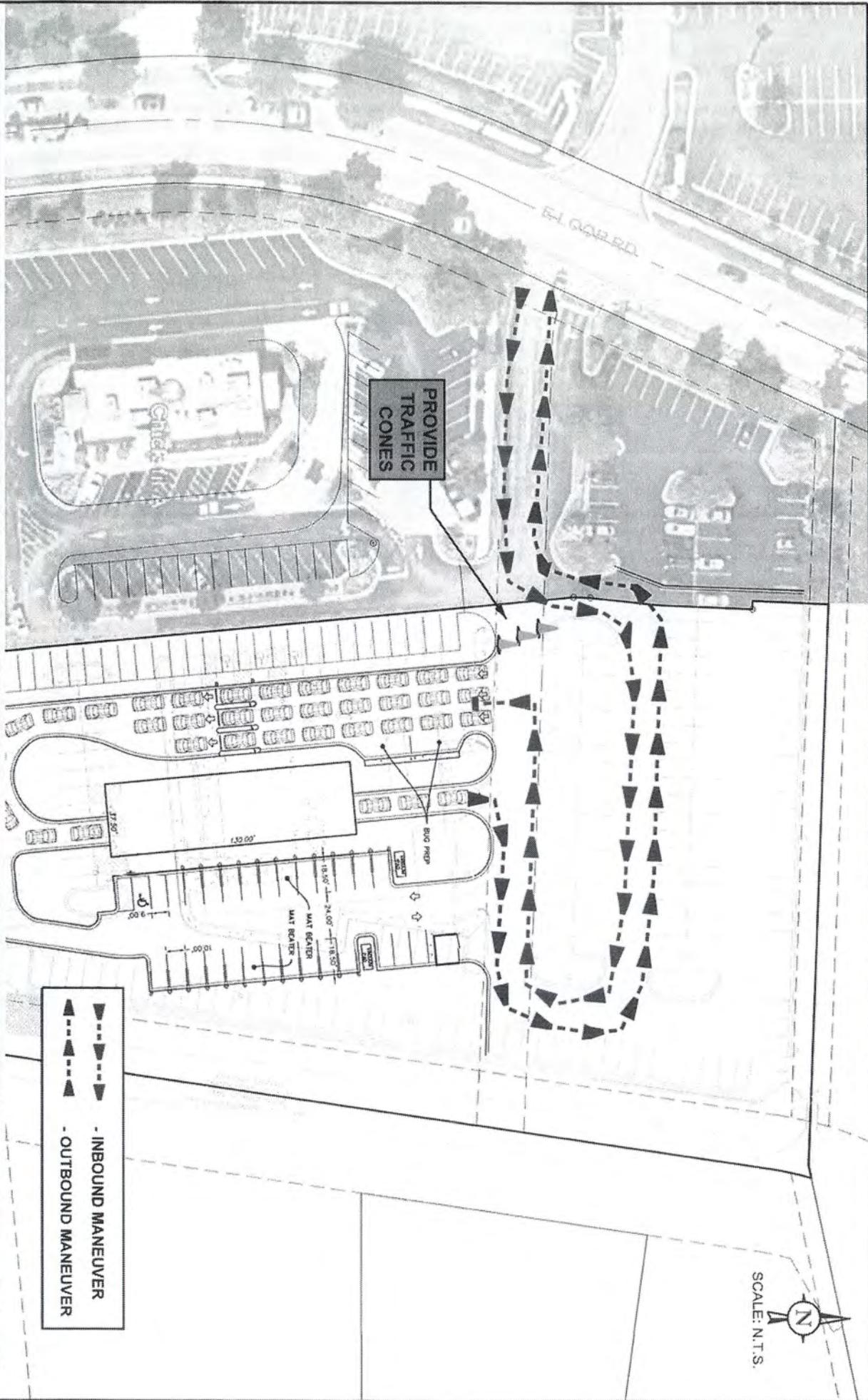
<b>MeritCorp</b>	
Engineering - Planning - Surveying - Environmental	4222 Mendota Parkway, Suite 112
Aurora, IL 60504	Office 630.554.6655
Lic. No. 184-005860	Other Office Locations: Gurnee, IL

CARWASH  
WHEATON, ILLINOIS

## PEAK TIMES PROPOSED TRAFFIC FLOW PATTERNS

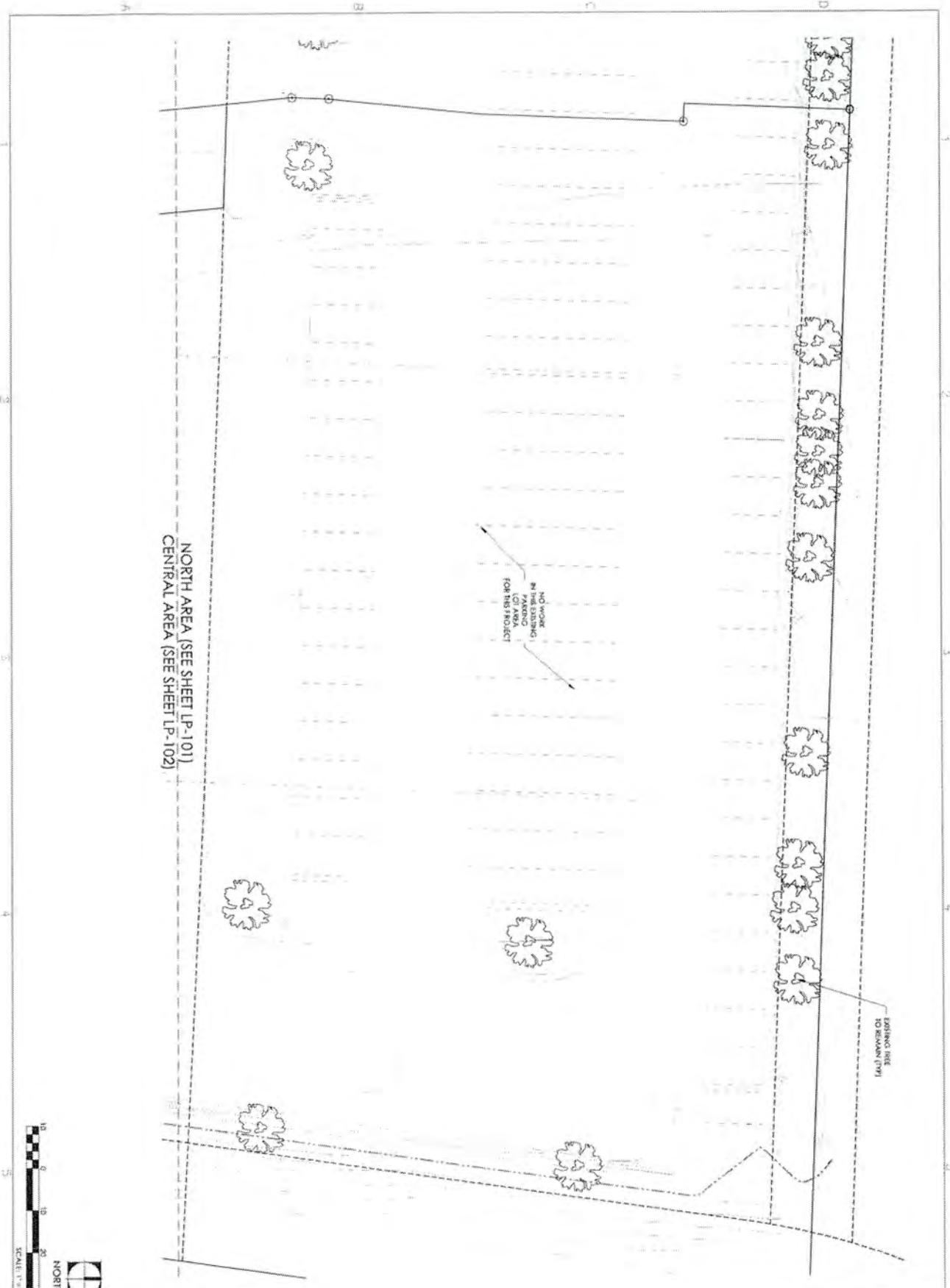
DRAWN:	ME	CHECKED:	JMM
DATE:	08-21-22	REV:	
PROJECT #22-163			
EXHIBIT: A			

**KLOA**  
Kenya Livingstone Overseas Agents, Ltd.









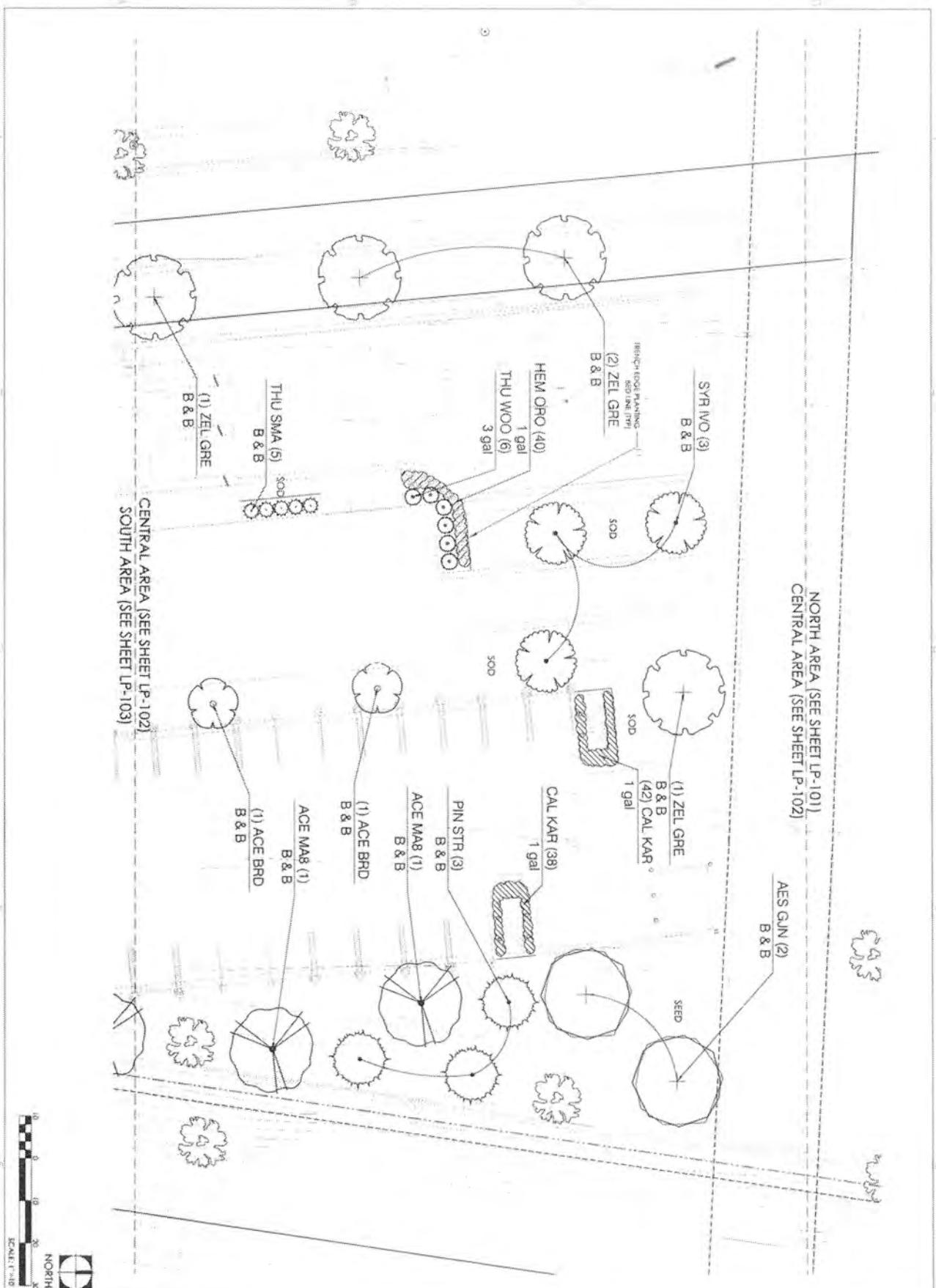
Wash U Car Wash  
311 East Loop Road  
Wheaton, IL 60189

**DesignPerspectives**  
Sensitizing to Design  
1167 Madison Mill Drive  
Kingsville, Illinois 60545  
Telephone: (847) 696-0776  
[www.dps.org/perspectives.html](http://www.dps.org/perspectives.html)



DATE: 7/1/2022  
JOB NO.: 22-3386  
DRAWN BY: TS  
CHECKED BY: TS  
DRAWING TITLE:  
LANDSCAPE PLAN,  
NORTH

卷之三



Wash U Car Wash  
311 East Loop Road  
Wheaton, IL 60189

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1147 Holden Ave, Denver  
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REV.	COMMENT	DATE

DATE: 7/1/2022  
JOB NO: 22-386  
DRAWN BY: TS  
CHECKED BY: TS  
DRAWN ON: 15  
LANDSCAPE PLAN  
CENTRAL

Sheet No.:  
LP-102

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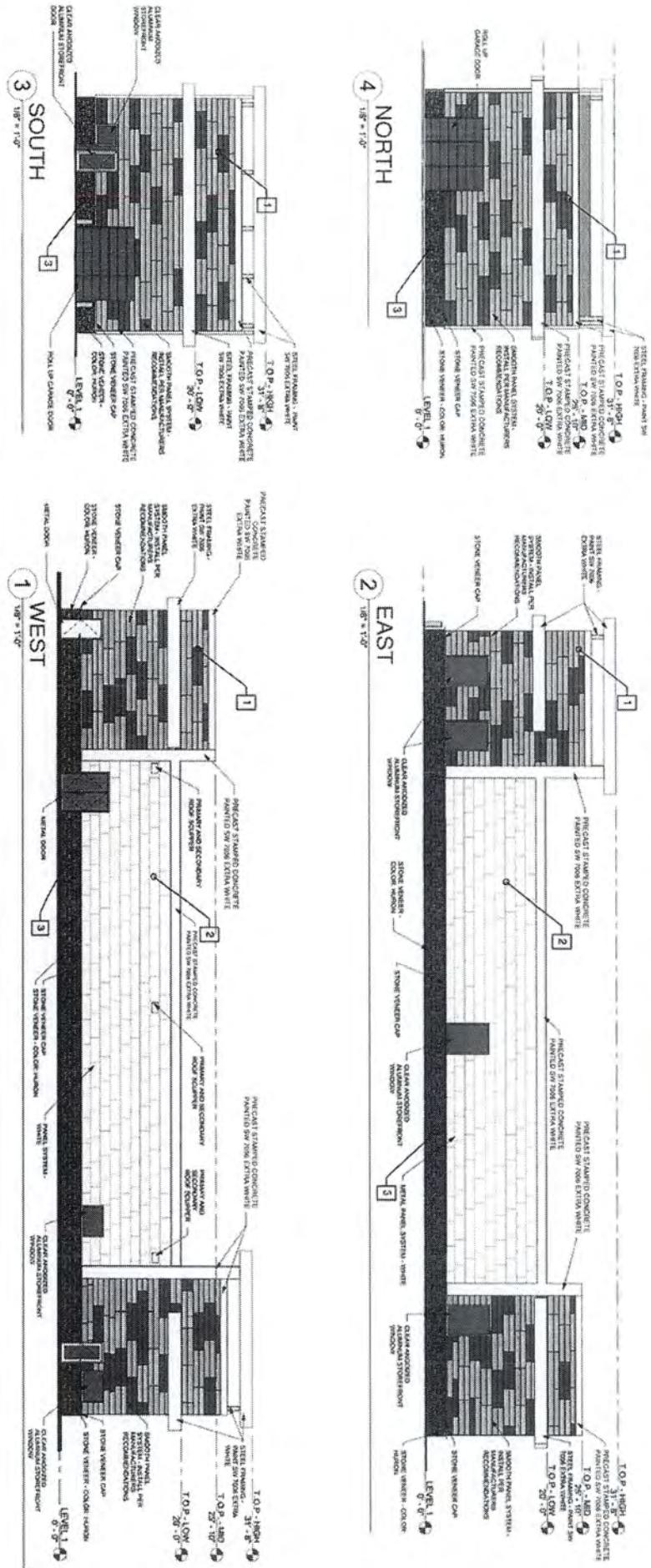




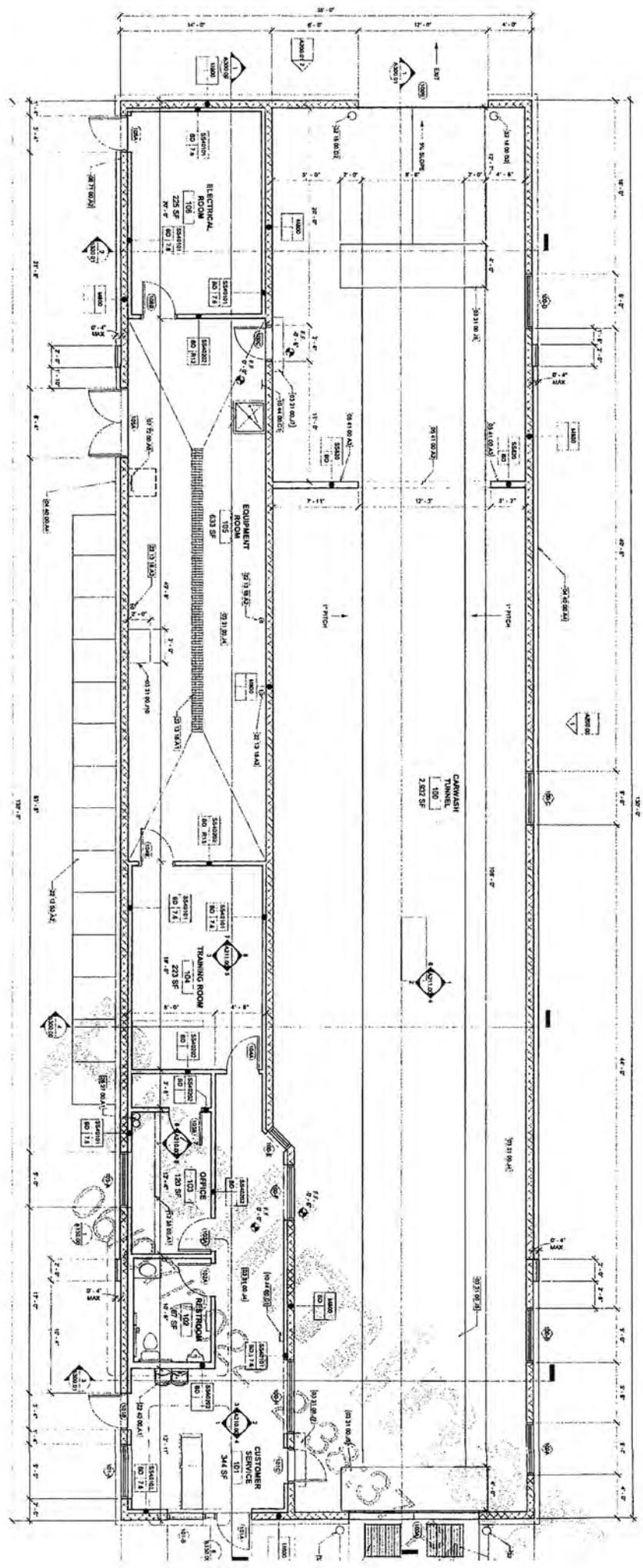




#### EXTERIOR MATERIALS LEGEND



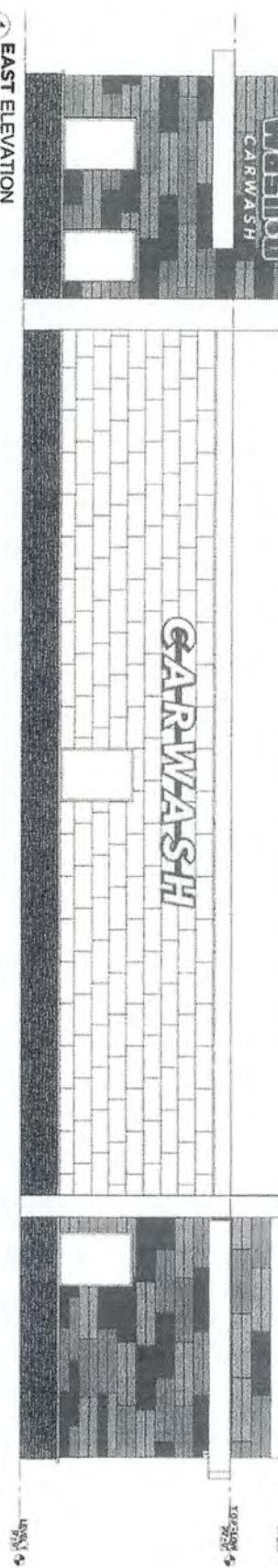
1 PROPOSED FLOOR PLAN



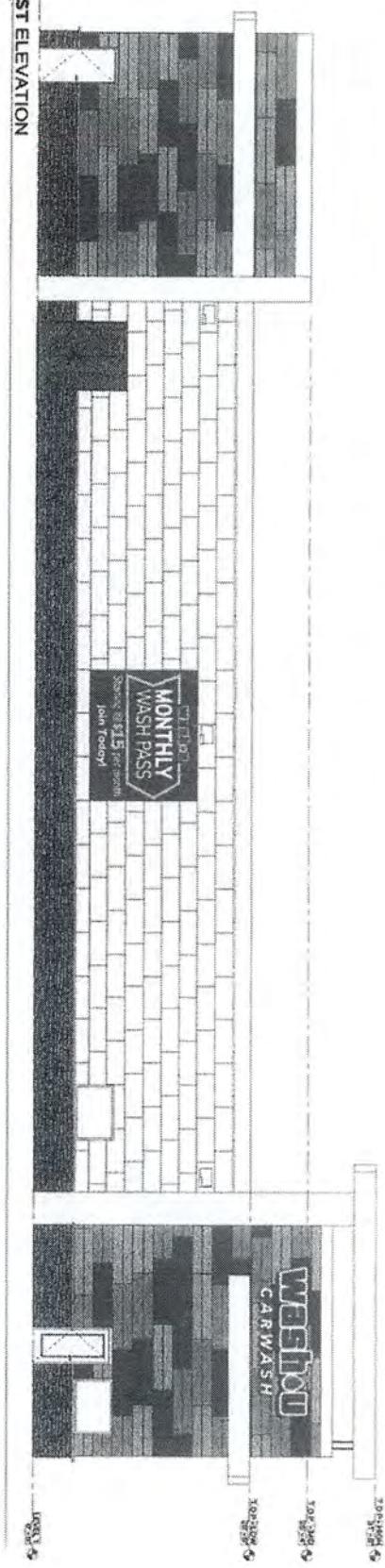
# Wash U



2 SOUTH ELEVATION



1 EAST ELEVATION



1 WEST ELEVATION

NATURE:

DATE:

DRAWING: wheaton exterior

MANAGER: B. Avink

DATE:

5/16/2022

20

• 1124 Electric Ave.

Wayland, MI 49348

• midwestsignco.com

**MIDWEST**  
SIGN COMPANY

PROJECT: Wash U/Wheaton II

DESIGNER: R. Sobota

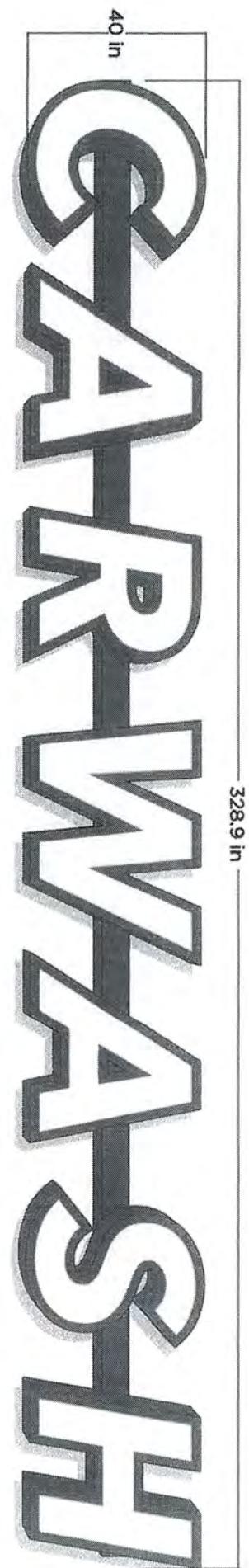
DATE:

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Wash U



68.75 SF, QTY (3)



91.4 SF

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Wayland, MI 49348  
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NATURE:

DATE:

DRAWING: wheaton exterior

MANAGER: B. Avink

DATE:

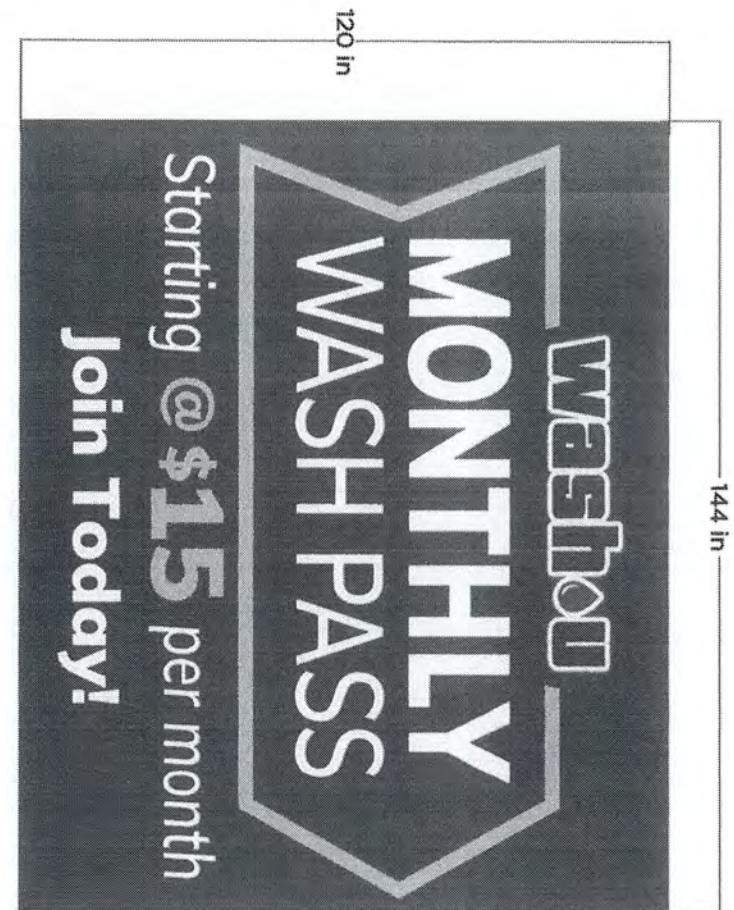
PROJECT: Wash U/Wheaton II

DESIGN: R. Sobota

5/16/2022

2.0

Wash U



1-sided 3 piece sign panel, 120 SF

;NATURE:		;DATE:	
DATE:		DRAWING: wheaton exterior	
PROJECT: Wash U/Wheaton IL		MANAGER: B. Avink	
DESIGN: R. Sobota		DATE:	
5/16/2022		2.0	
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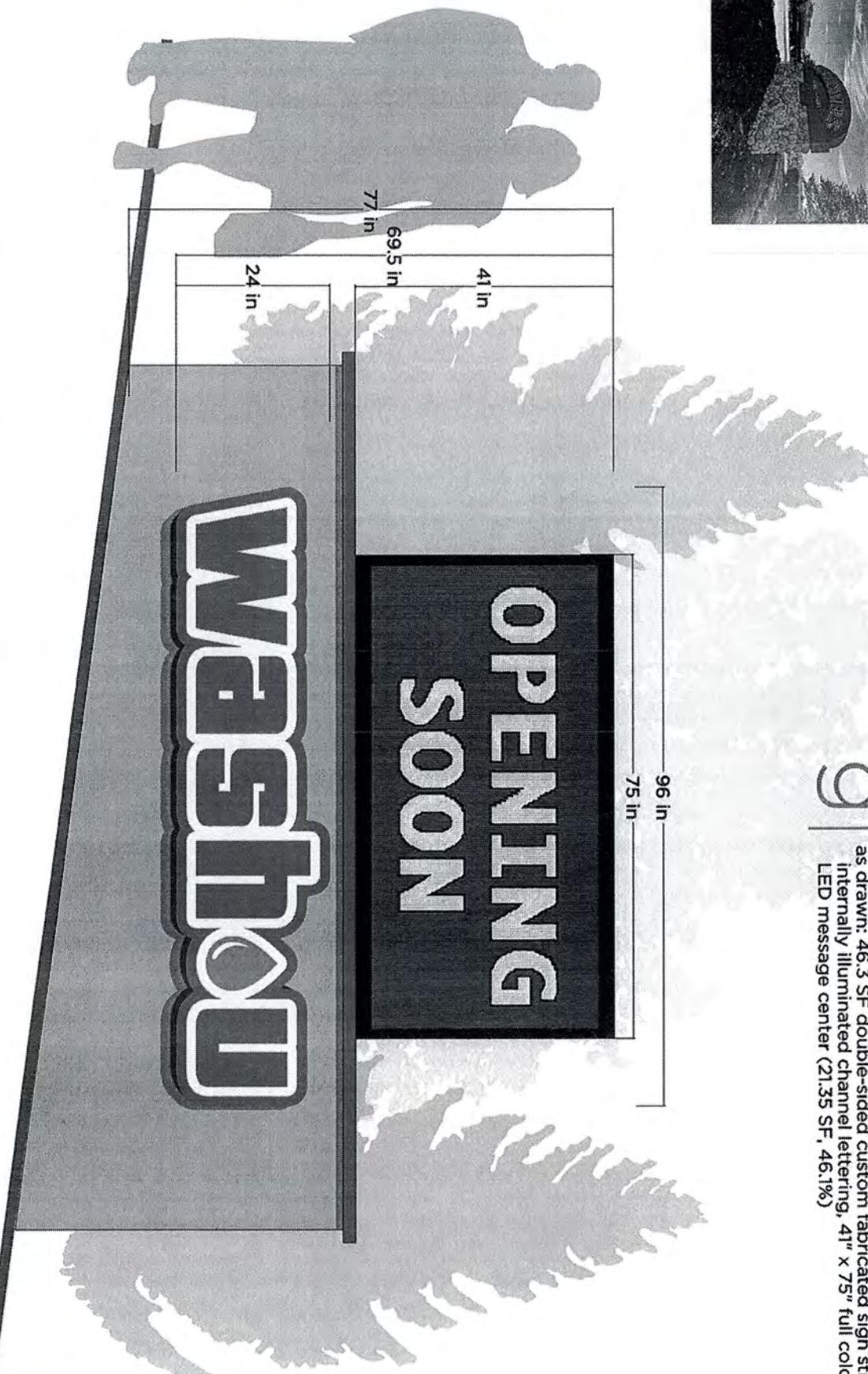
Wash U



9

criteria: 48 SF maximum, 7 FT height maximum, 50% EMC maximum  
as drawn: 46.3 SF double-sided custom fabricated sign structure with  
internally illuminated channel lettering, 41" x 75" full color Watchfire  
LED message center (21.35 SF, 46.1%)

OPENING  
SOON



SIGNATURE:

DATE:

DRAWING: wheaton main id revB

MANAGER: B. Avink

DATE:

PROJECT: Wash U/Wheaton, IL

DESIGN: R. Sobota

DATE:

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Wayland, MI 49348

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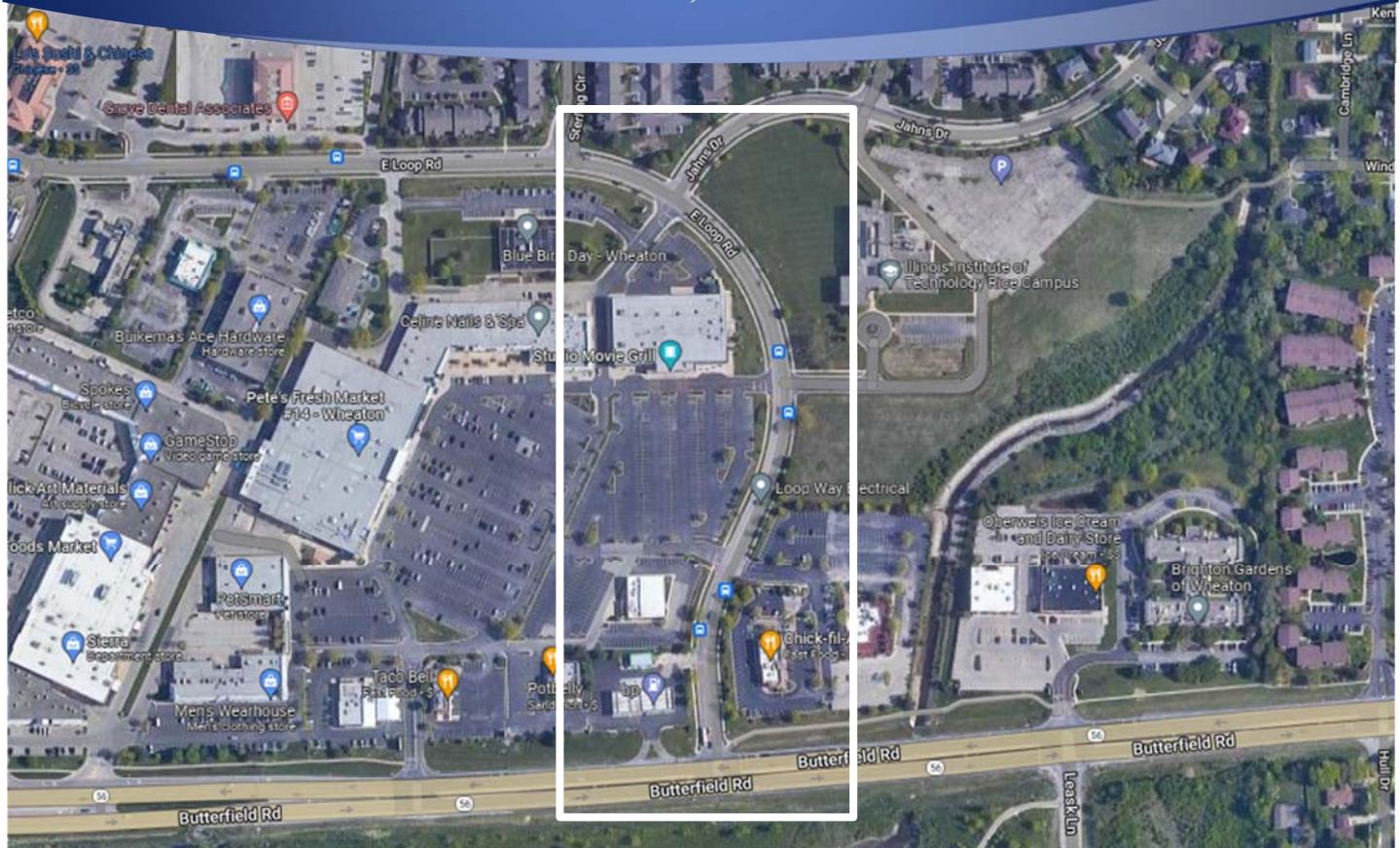
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# Comprehensive Traffic Study

## North Section of East Loop Road

Wheaton, Illinois



Prepared For:



**KLOA**  
Kenig, Lindgren, O'Hara, Aboona, Inc.

February 23, 2023

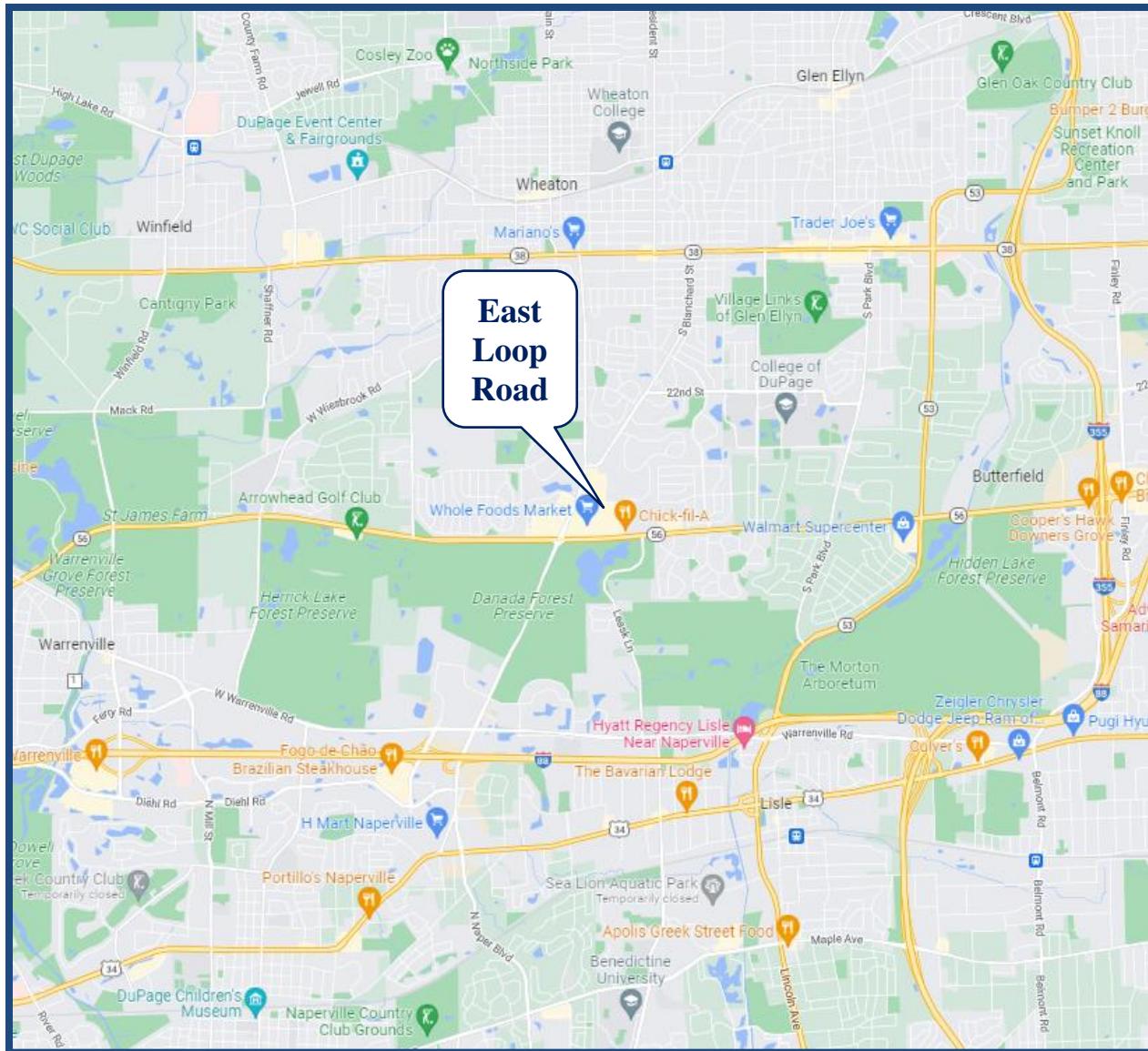
# 1. Introduction

This report presents the methodologies, findings, and recommendations of a comprehensive traffic study conducted by Kenig, Lindgren, O'Hara, Aboona, Inc. (KLOA, Inc.) for the north-south section of East Loop Road located in Wheaton, Illinois. East Loop Road is an approximate 0.6-mile road that extends in a quarter circle between IL 56 and Naperville Road. The road has a three-lane cross-section and is under traffic signal control at its intersections with IL 56 and Naperville Road. **Figure 1** shows the location of East Loop Road in relation to the area roadway system. **Figure 2** shows an aerial view of the East Loop Road corridor.

East Loop Road provides access to the Danada Square subdivision and serves several commercial, office, institutional, and multi-family uses located north of IL 56 and east of Naperville Road. In addition, the road serves as a by-pass route around the major intersection of IL 56 with Naperville Road. The volume of traffic along East Loop Road, particularly the southern section, is projected to increase with the approval of the Wash U car wash and the full occupancy of the Rice Lake Square shopping center. As a result, several transportation issues and concerns have been raised regarding the existing and projected operations of East Loop Road and the roads and access drives intersecting the road. As such, the City of Wheaton has requested that a comprehensive traffic study be performed of the north-south section of East Loop Road.

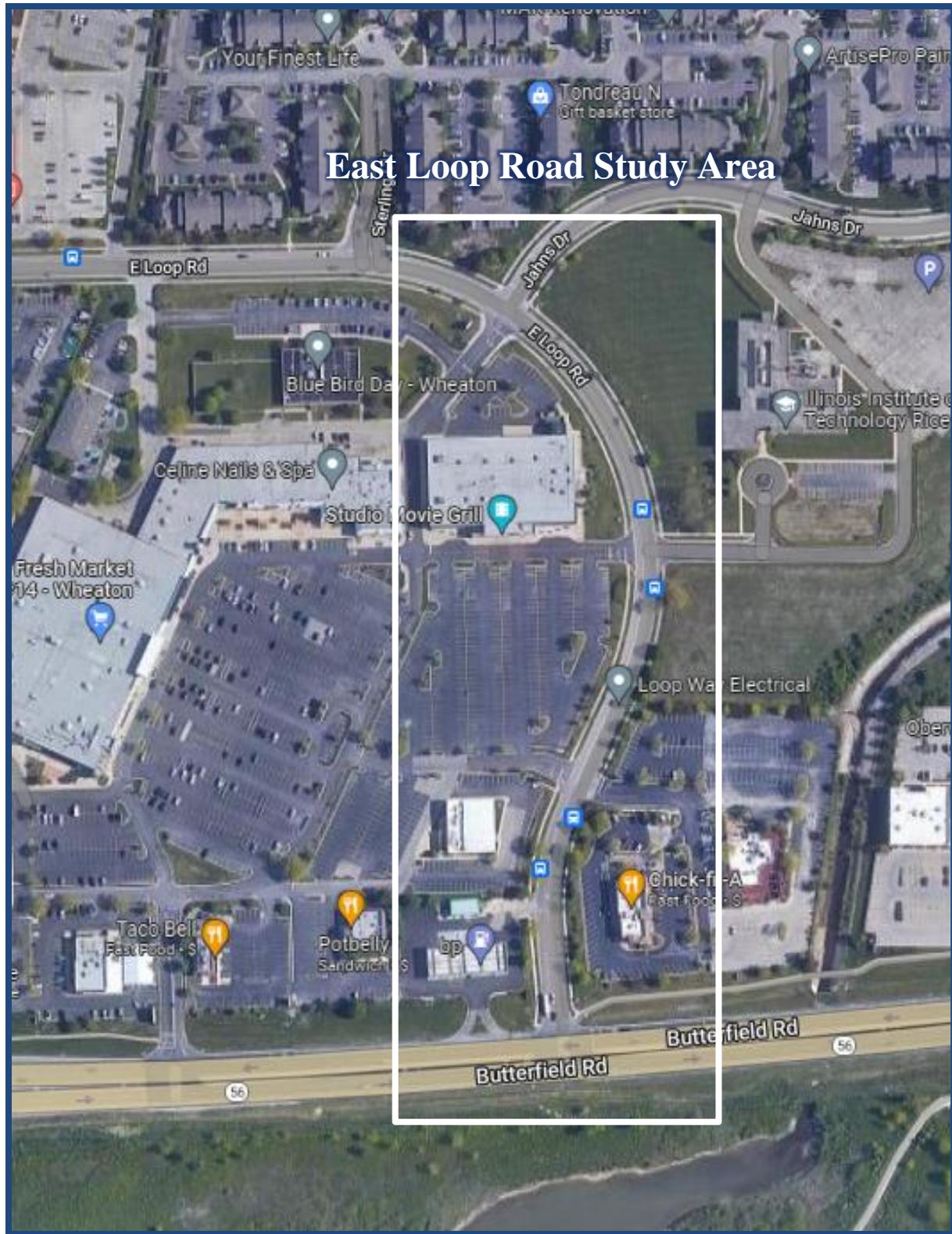
Through this study, KLOA, Inc. will thoroughly document the existing roadway conditions, estimated the projected traffic volumes based on the projected growth in the area, evaluate the existing and projected operations, examine alternative improvements and/or modifications, and, if necessary, develop recommendations to enhance the operations of the corridor. Per the direction of the City of Wheaton, the traffic study focused on the north-south section of East Loop Road, which extends between IL 56 and Jahns Drive, and includes the following four intersections:

- East Loop Road with IL 56
- East Loop Road with Jahns Drive and the Rice Lake Square northern access drive
- East Loop Road with the Rice Lake Square southern access drive and the Chick-fil-A access drive
- East Loop Road with the Rice Lake Square middle access drive and the Illinois Institute of Technology access drive



**Corridor Location**

**Figure 1**



Aerial View of Corridor

Figure 2

## 2. Existing Conditions

Existing transportation conditions along East Loop Road and the crossroads and access drives were documented based on field visits conducted by KLOA, Inc. in order to obtain a database for projecting future conditions. The following provides a description of the geographical location of the corridor, physical characteristics of the area roadway system including lane usage and traffic control devices, and existing peak hour traffic volumes.

### Existing Land Uses

As previously noted, East Loop Road provides access to the Danada Square subdivision and serves several commercial, office, institutional, and multi-family uses located north of IL 56 and east of Naperville Road. Multi-family developments are located north of Jahns Drive and the east-west section of East Loop Road. The following summarizes the specific existing land uses located along the north-south section of East Loop Road:

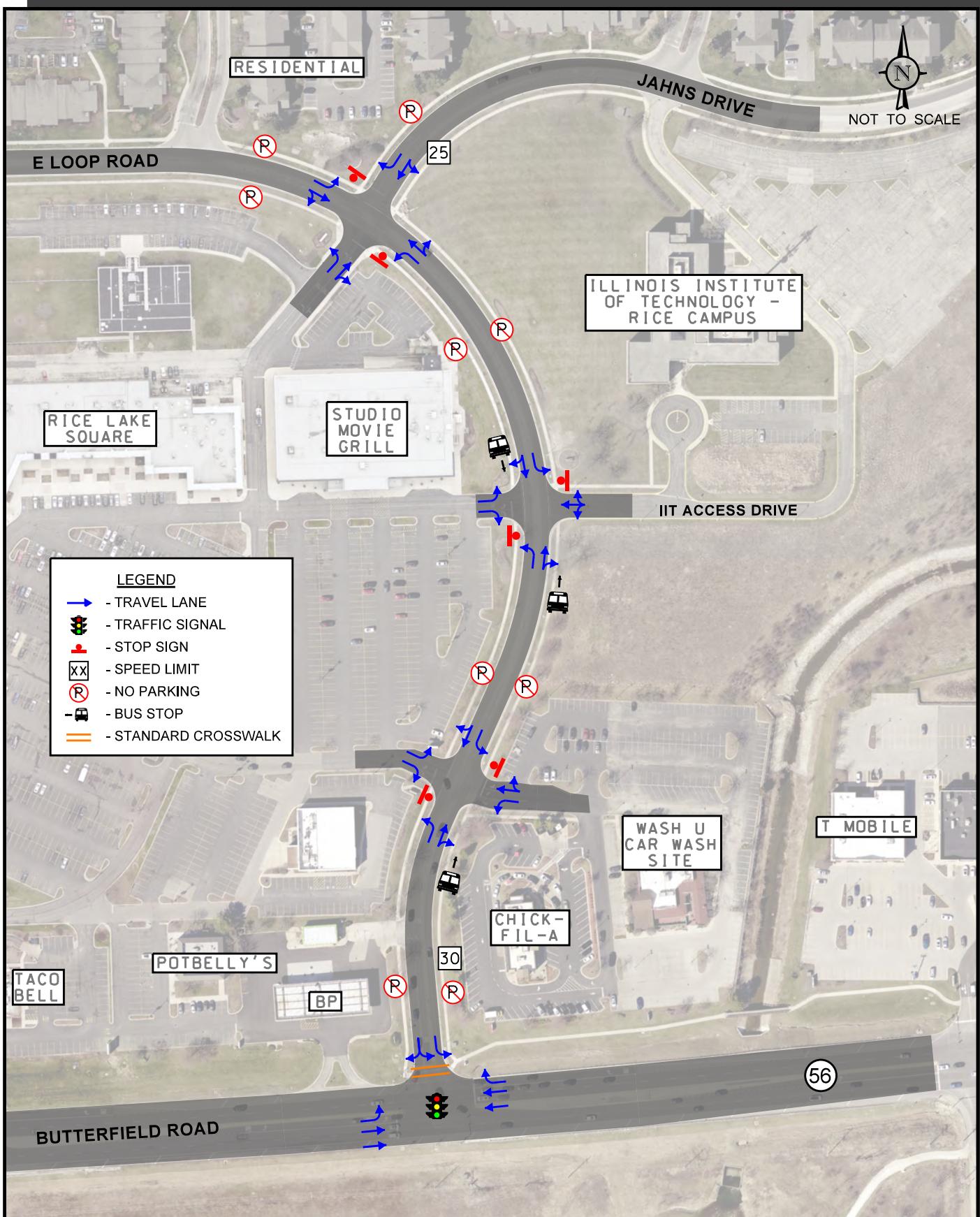
- A Chick-fil-A restaurant and the Wash U car wash site are located in the northeast quadrant of the East Loop Road/IL 56 intersection.
- The Illinois Institute of Technology campus (IIT) is located in the southeast quadrant of the East Loop Road/Jahns Drive intersection.
- A BP fuel center is located in the northwest quadrant of the East Loop Road/IL 56 intersection.
- The Rice Lake Square shopping center is located on the west side of East Loop Road bounded by IL 56 on the south and the east-west section of East Loop Road on the north.

### Existing Roadway System Characteristics

The characteristics of East Loop Road and the adjacent roadways and access drives within the study area are described below and illustrated in **Figure 3**.

*East Loop Road* is an approximate 0.6-mile major collector road that extends in a quarter circle between its signalized intersection with IL 56 and its signalized intersection with Naperville Road. The road has one lane in each direction with a center median that accommodates separate left-turn lanes serving the various crossroads and access drives along the road. East Loop Road is under the jurisdiction of the City of Wheaton and has a posted speed limit of 30 miles per hour. The following summarizes the lane geometrics provided along East Loop Road at the various intersections:

- At its signalized intersection with IL 56, East Loop Road has a separate left-turn lane and a shared left-turn/right-turn lane on the southbound approach.
- At its unsignalized intersections with (1) Jahns Drive and the Rice Lake Square northern access drive, (2) the Rice Lake Square southern access drive and the Chick-fil-A access drive, and (3) the Rice Lake Square middle access drive and the Illinois Institute of Technology access drive, East Loop Road has a separate left-turn lane and a shared through/right-turn lane on both approaches.



*IL 56 (Butterfield Road)* is an east-west, principal arterial roadway that has two travel lanes in each direction. The road is under the jurisdiction of the Illinois Department of Transportation (IDOT), is classified as a Strategic Regional Arterial (SRA) route, carries an Annual Average Daily Traffic (AADT) volume of 24,700 vehicles (IDOT 2021), and has a posted speed limit of 45 miles per hour. At its signalized intersection with East Loop Road, IL 56 has a separate left-turn lane and two through lanes on the eastbound approach and two through lanes and a separate right-turn lane on the westbound approach.

*Jahns Drive* is a local road that extends north and east from East Loop Road to Hawkins Circle. It has one lane in each direction and is aligned opposite the Rice Lake Square northern access drive at its intersection with East Loop Road. At its unsignalized intersection with East Loop Road, Jahns Drive has a shared left-turn/through lane and a separate right-turn lane that are under stop sign control.

*Access Drives.* The following summarizes the characteristics of the access drives that intersect East Loop Road within the study area:

- The Chick-fil-A access drive has one inbound lane and two outbound lanes striped for a separate left-turn lane and a shared through/right-turn lane with the outbound lanes under stop sign control.
- The Illinois Institute of Technology access drive has one inbound lane and one outbound lane with the outbound lane under stop sign control.
- The Rice Lake Square southern and middle access drives both have one inbound lane and two outbound lanes striped for a separate left-turn lane and a separate right-turn lane with the outbound lanes under stop sign control.
- The Rice Lake Square northern access drive has one inbound lane and two outbound lanes striped for a separate left-turn lane and a shared through/right-turn lane with the outbound lanes under stop sign control.

## East Loop Road Alternative Modes of Transportation

The following summarizes the alternative modes of transportation that extend along East Loop Road and the other roads in the study area:

- Sidewalks are located along both sides of East Loop Road and Jahns Drive.
- Both East Loop Road and Jahns Drive are designated bike routes.
- Pace Bus Route 714 runs along East Loop Drive.

## Existing Traffic Volumes

To determine current traffic conditions in the vicinity of the site, KLOA, Inc. conducted peak period vehicle, pedestrian, bicycle counts using Miovision Scout Collection Units at the following intersections:

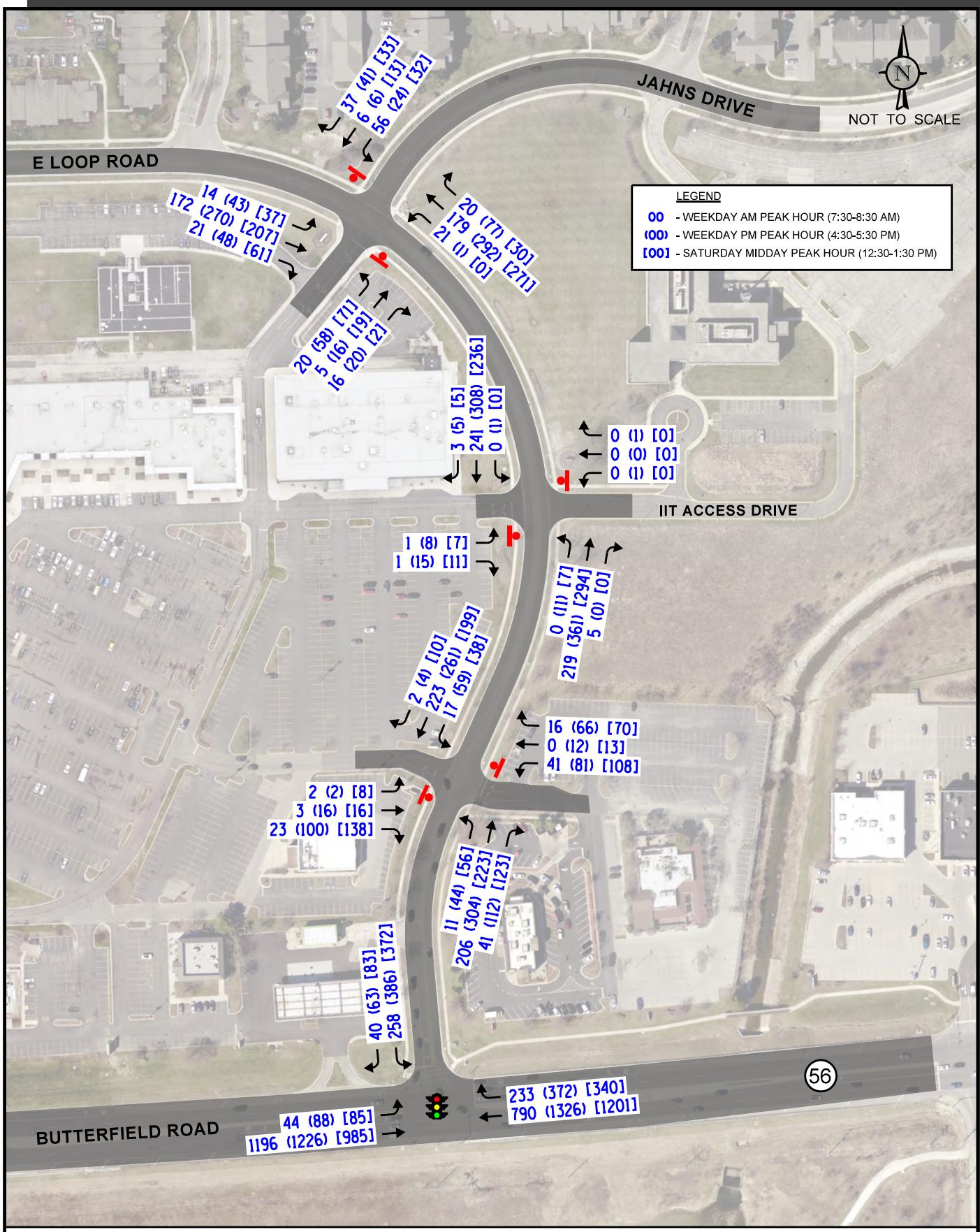
- East Loop Road with IL 56 (Tuesday and Saturday, December 6 and 10, 2022)
- East Loop Road with Jahns Drive and the Rice Lake Square northern access drive (Thursday and Saturday, June 2 and 4, 2022)
- East Loop Road with the Rice Lake Square southern access drive and the Chick-fil-A access drive (Tuesday and Saturday, December 6 and 10, 2022)
- East Loop Road with the Rice Lake Square middle access drive and the Illinois Institute of Technology access drive (Tuesday and Saturday, December 6 and 10, 2022)

The traffic counts were performed during the weekday morning (7:00 A.M. to 9:00 A.M.), weekday evening (4:00 P.M. to 6:00 P.M.), and Saturday midday (11:30 A.M. to 1:30 P.M.) peak periods. It should be noted that the traffic counts were performed during the peak holiday season for the adjacent commercial developments. The results of the traffic counts showed that the weekday morning peak hour of traffic occurs from 7:30 A.M. to 8:30 A.M., the weekday evening peak hour of traffic occurs from 4:30 P.M. to 5:30 P.M., and the Saturday midday peak hour of traffic occurs from 12:30 P.M. to 1:30 P.M. **Figure 4** illustrates the existing peak hour traffic volumes. Copies of the traffic count summary sheets are included in the Appendix.

## Field Observations

As part of the study, KLOA, Inc. performed field observations and reviewed the videos taken as part of the traffic counts conducted at the intersections in the study area. Both the videos collected as part of the December 2022 traffic counts and the traffic counts performed in June 2022 as part of the Wash U traffic study were reviewed. The results of the field observations and the review of the videos revealed the following:

- Overall, the East Loop Road corridor generally operates well with limited congestion. Some delay and queueing can occur along the corridor. However, any queueing generally dissipates quickly and is typical of collector roads serving commercial areas.
- The southbound approach of East Loop Road at its intersection with IL 56 generally operates well and all vehicles typically clear the intersection during each green phase. Further, a review of the videos as well as field observations revealed that the southbound queues along East Loop Road at its intersection with IL 56 rarely extend past the Rice Lake Square southern access drive and the Chick-fil-A access drive. The few times that the queue was observed extending past the access drives, the queue quickly dissipated.



- Jahns Drive and all the access drives in the study area generally operate well. A review of the videos as well as the field observations showed that the Chick-fil-A access drive has an average queue of one to two vehicles with a typical peak queue of three to four vehicles. The queue exceeded four vehicles on several occasions. However, it is important to note that the queues generally dissipated quickly.

## Crash Data Summary

KLOA, Inc. obtained crash data<sup>1</sup> for the most recent available past five years (2017 to 2021) for the four intersections in the study area. The crash data for the intersection of IL 56 with East Loop Road is summarized in **Table 1** and the following summarizes the crash data at the other three intersections:

- At the East Loop Road/Jahns Drive/Rice Lake Square northern access drive intersection, two crashes were reported in 2017 and 2019, one crash was reported in 2018, and zero crashes were reported in 2020 and 2021.
- At the East Loop Road/Rice Lake Square southern access drive/Chick-fil-A access drive intersection, one crash was reported in 2020.
- At the East Loop Road/Rice Lake Square northern access drive/Illinois Institute of Technology intersection, no crashes were reported over the five-year period.

Table 1  
IL 56 WITH EAST LOOP ROAD – CRASH SUMMARY

Year	Type of Crash Frequency							
	Angle	Head On	Object	Rear End	Sideswipe	Turning	Other	Total
2017	0	0	1	2	0	2	0	5
2018	0	0	0	1	0	3	0	4
2019	0	0	0	1	0	3	0	4
2020	0	0	0	1	1	1	0	3
2021	0	0	0	2	0	3	0	5
<b>Total</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>7</b>	<b>1</b>	<b>12</b>	<b>0</b>	<b>21</b>
<b>Average</b>	<b>0</b>	<b>0</b>	<b>&lt; 1</b>	<b>1.4</b>	<b>&lt; 1</b>	<b>2.4</b>	<b>0</b>	<b>4.2</b>

<sup>1</sup> IDOT DISCLAIMER: The motor vehicle crash data referenced herein was provided by the Illinois Department of Transportation. Any conclusions drawn from analysis of the aforementioned data are the sole responsibility of the data recipient(s). Additionally, for coding years 2015 to present, the Bureau of Data Collection uses the exact latitude/longitude supplied by the investigating law enforcement agency to locate crashes. Therefore, location data may vary in previous years since data prior to 2015 was physically located by bureau personnel.

### 3. Year 2028 Projected Traffic Volumes

In order to determine how the East Loop Road corridor will operate in the future, Year 2028 traffic projections were developed that included the traffic to be generated by (1) regional (ambient) traffic growth and (2) any specific developments along the corridor.

#### Regional (Ambient) Traffic Growth

The existing traffic volumes (Figure 4) were increased by a regional growth factor to account for the increase in existing traffic related to regional growth in the area (i.e., not attributable to any particular planned development). Based on ADT projections provided by the Chicago Metropolitan Agency for Planning (CMAP) in a letter dated January 5, 2023, the existing traffic volumes were increased by an annually compounded growth rate of 0.5 percent per year for six years (one-year buildout plus five years) for a total of approximately three percent to represent Year 2028 background conditions. A copy of the CMAP projections letter is included in the Appendix.

#### Corridor Developments

Per the direction of the City of Wheaton, the study also included the traffic to be generated by the following existing developments or proposed developments:

- The occupancy of the vacant space in the Rice Lake Square shopping center
- The reestablishment of a technical institute/school at the IIT facility
- The approved Wash U car wash to be located in the northeast quadrant of the IL 56/East Loop Road intersection
- The potential redevelopment of the parking lot located directly north of the Chick-fil-A restaurant and approved Wash U car wash with a 7,000 square-foot sit-down restaurant.

#### Rice Lake Square Shopping Center

According to the City, the Rice Lake Square shopping center has approximately 68,800 square feet of vacant commercial space. The traffic to be generated by the occupancy of the vacant space was estimated based on the existing traffic generated by the shopping center and assigned to the existing access drives and roadway system based on the existing traffic conditions. It is important to note that access to the Rice Lake Square shopping center is provided via three access drives on East Loop Road, one access drive on IL 56, and cross-access to the developments located east of the shopping center. As such, not all the traffic to be generated by the vacant space will be using East Loop Road.

## IIT Facility

The currently closed IIT facility is located in the southeast quadrant of East Loop Road with Jahns Drive. Access to the facility is provided via one access drive on East Loop Road and one access drive on Jahns Drive. The traffic to be generated by the reestablishment of a technical institute/school at the facility was based on the number of parking spaces serving the facility and the arrival/departure of students/staff and was assigned to the existing access drives and roadway system based on the existing traffic conditions. It should be noted that if the IIT facility is redeveloped as multi-family homes, similar to the homes located directly north of the IIT site, the multi-family homes will generate significantly lower weekday morning and evening peak hour traffic volumes compared to the projected traffic to be generated by the reestablishment of the IIT facility as a technical institute/school.

## Wash U Car Wash

### *Development Plan*

The Wash U car wash is to replace the approximate 12,000 square-foot vacant restaurant located on the north side of IL 56 directly east of the Chick-fil-A restaurant. As approved, the car wash is to contain an automatic car wash tunnel with six employee parking spaces and 22 vacuum parking spaces. As was the case with the existing restaurant, access to the car wash will be provided via the existing East Loop Road access drive that serves both the Chick-fil-A restaurant and the restaurant site.

### *Trip Generation Estimates*

The number of peak hour trips estimated to be generated by the car wash was based on “Automated Car Wash” (Land-Use Code 948) vehicle trip generation rates contained in *Trip Generation Manual*, 11<sup>th</sup> Edition, published by ITE. It should be noted that the *Trip Generation Manual* does not provide data for the morning peak hour for an automated car wash. Therefore, the weekday morning peak hour traffic to be generated by the car wash was assumed to be approximately one-third of the weekday evening peak hour trip generation.

It is important to note that surveys conducted by ITE have shown that a significant number of trips made to car washes are diverted from the existing traffic on the roadway system. This is particularly true during the weekday morning and evening peak hours when traffic is diverted from the home-to-work and work-to-home trips. Such diverted trips are referred to as pass-by traffic. However, in order to present a worst-case scenario, no reduction in the development trip generation estimates was considered. Additionally, the trip generation estimates were not reduced to account for the potential interaction between the car wash and the Chick-fil-A restaurant as well as the proximity to the Rice Lake Square shopping center.

**Table 2** summarizes the estimated peak hour trips to be generated by the car wash on a typical or normal day. The estimated traffic volumes were assigned to the roadway system based on the operation of the existing roadway system as determined by the existing traffic volumes. The directional distribution for the car wash is illustrated in **Figure A**, located in the Appendix.

Table 2  
CAR WASH ESTIMATED PEAK HOUR TRAFFIC VOLUMES

ITE Land-Use Code	Type	Weekday Morning Peak Hour			Weekday Evening Peak Hour			Saturday Midday Peak Hour		
		In	Out	Total	In	Out	Total	In	Out	Total
948	Car Wash (1 tunnel)	13	13	26	39	39	78	19	22	41

Lastly, the trip generation shown in Table 2 represents the traffic to be generated by the car wash on a typical or normal day. It should be noted that peak operations (design day) of a car wash generally occur after a weather event such as a snowfall or a rain event. Based on historical data from other car washes, the peak days at a car wash normally occur 12 to 15 times per year. Since the peak days at the car wash only occur approximately five percent of the days per year, the peak day volumes were not evaluated as part of this study.

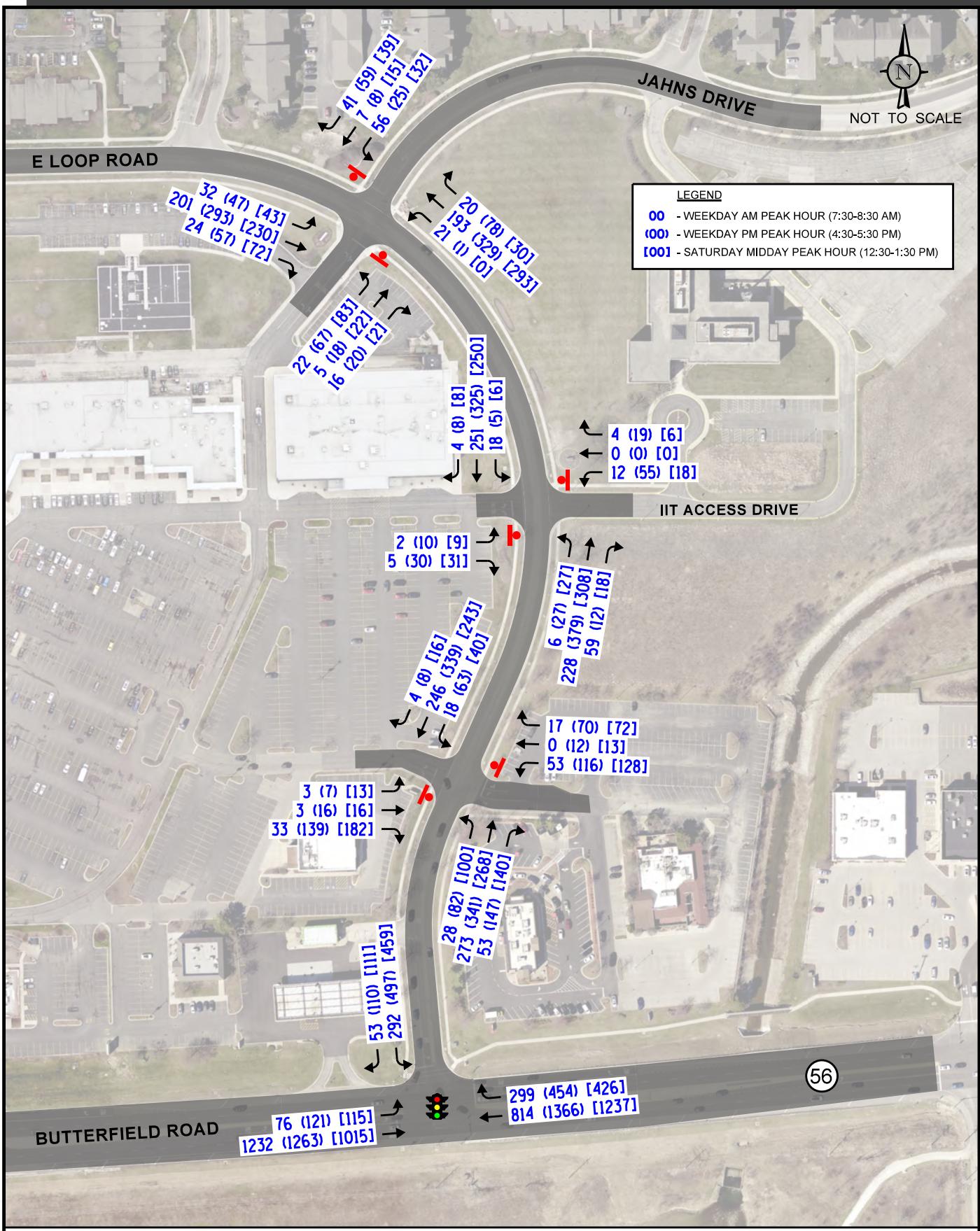
## Potential 7,000 Square-Foot Sit-Down Restaurant

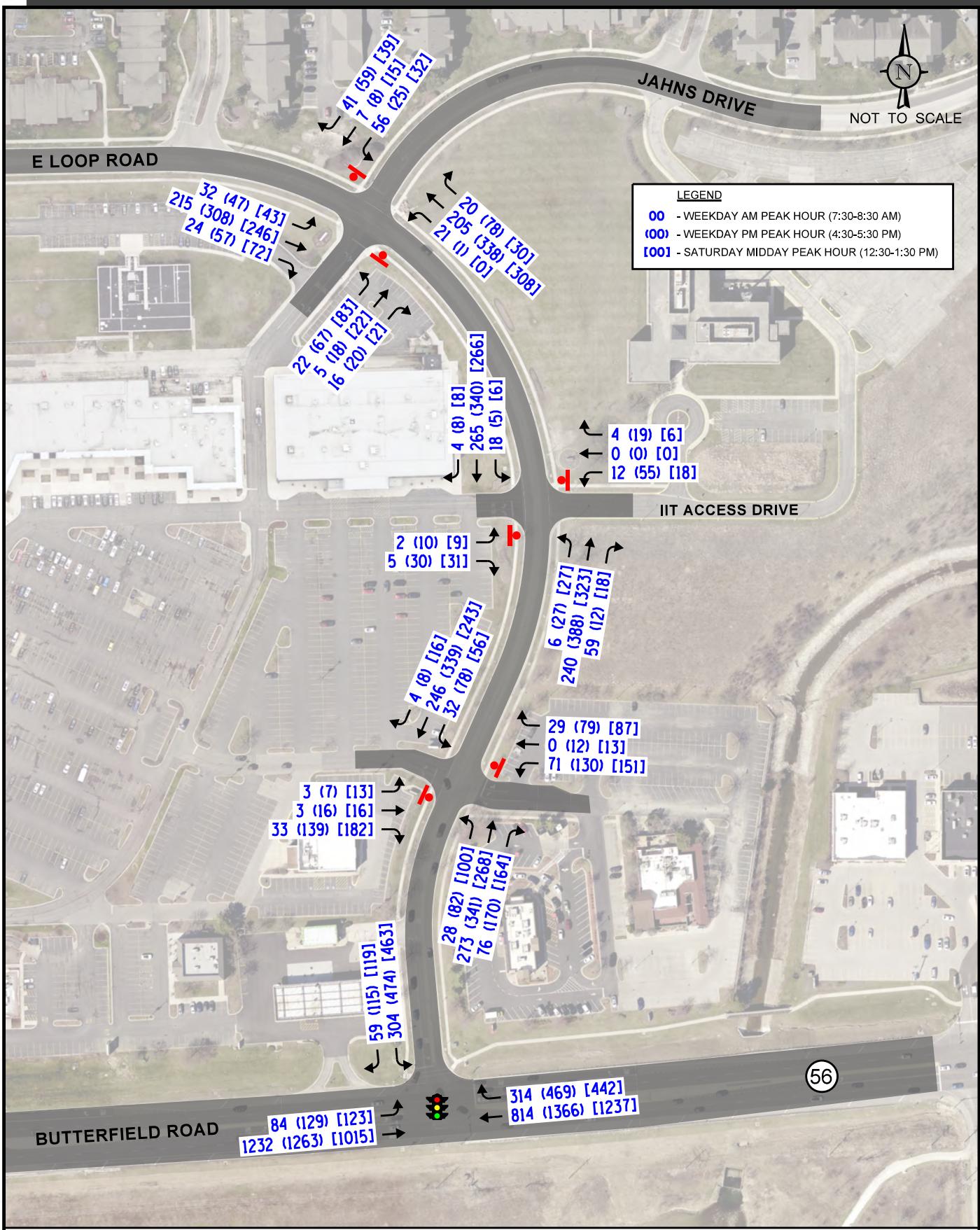
In addition to the occupancy/reestablishment of the existing vacant stores/buildings along the corridor, the City of Wheaton also requested that the study examine the potential redevelopment of the existing parking lot located directly north of the Chick-fil-A restaurant and the Wash U car wash. It should be noted that any redevelopment of the parcel will require a zoning change. Per the direction of the City of Wheaton, the parking lot was assumed to be redeveloped with a 7,000 square-foot sit-down restaurant. The volume of traffic to be generated by the restaurant was based on the trip rates provided in the *ITE Trip Generation Manual, 5<sup>th</sup> Edition* and assigned to the existing access drive and roadway system based on the existing traffic conditions.

## Year 2028 Projected Traffic Volumes

The following two Year 2028 traffic assignments were developed and evaluated:

- The *Year 2008 Traffic Volumes without the Potential Sit-Down Restaurant* which include the existing traffic volumes, the traffic to be generated by the Wash U car wash, the full occupancy of the Rice Lake Square shopping center, and the occupancy of the IIT facility as well as the regional (ambient) growth are illustrated in **Figure 5**.
- The *Year 2008 Traffic Volumes with the Potential Sit-Down Restaurant* which include the existing traffic volumes, the traffic to be generated by the Wash U car wash, the full occupancy of the Rice Lake Square shopping center, the occupancy of the IIT facility, and the traffic to be generated by the potential 7,000 square-foot sit-down restaurant as well as the regional (ambient) growth are illustrated in **Figure 6**.





## 4. Traffic Analysis and Recommendations

The following provides an evaluation conducted for the weekday morning, weekday evening, and Saturday midday peak hours. The analysis includes conducting capacity analyses to determine how well the roadway system and access drives currently operate and are projected to operate and whether any roadway improvements or modifications are required.

### Traffic Analyses

Roadway and adjacent or nearby intersection analyses were performed for the weekday morning, weekday evening, and Saturday midday peak hours peak hours for the Year 2022 existing and Year 2028 total conditions.

The traffic analyses were performed using the methodologies outlined in the Transportation Research Board's *Highway Capacity Manual* (HCM), 6<sup>th</sup> Edition and analyzed using Synchro/SimTraffic 11 software. The analysis for the traffic-signal controlled intersections were accomplished using actual and field measured lengths and phasings to determine the average overall vehicle delay and levels of service.

The analyses for the unsignalized intersections determine the average control delay to vehicles at an intersection. Control delay is the elapsed time from a vehicle joining the queue at a stop sign (includes the time required to decelerate to a stop) until its departure from the stop sign and resumption of free flow speed. The methodology analyzes each intersection approach controlled by a stop sign and considers traffic volumes on all approaches and lane characteristics.

The ability of an intersection to accommodate traffic flow is expressed in terms of level of service, which is assigned a letter from A to F based on the average control delay experienced by vehicles passing through the intersection. The *Highway Capacity Manual* definitions for levels of service and the corresponding control delay for signalized intersections and unsignalized intersections are included in the Appendix of this report.

Summaries of the traffic analysis results showing the level of service and overall intersection delay (measured in seconds) for the Year 2022 existing and Year 2028 conditions are presented in **Tables 4 through 7**. A discussion of each intersection follows. Summary sheets for the capacity analyses are included in the Appendix.

Table 4  
CAPACITY ANALYSIS RESULTS – SIGNALIZED  
IL 56 WITH EAST LOOP ROAD

Weekday Morning Peak Hour	Condition	Eastbound		Westbound		Southbound L/R	Overall		
		L	T	T	R				
Weekday Morning Peak Hour	Year 2022 Existing Conditions	A 4.7	A 7.3	A 9.6	A 9.2	E 56.9	B 13.9		
	A – 7.2		A – 9.5						
	Year 2028 Total Conditions <sup>1</sup>	A (A) 5.9 (6.3)	A (A) 8.5 (8.9)	B (B) 11.8 (12.3)	B (B) 11.3 (12.6)	E (E) 55.9 (55.6)	B (B) 15.7 (16.3)		
Weekday Evening Peak Hour	A – 8.4 (A – 8.7)		B – 11.8 (B – 12.4)						
	Year 2022 Existing Conditions	A 8.8	A 9.1	B 15.6	B 13.8	D 54.4	B 17.9		
	A – 9.0		B – 15.2						
Weekday Evening Peak Hour	Year 2028 Total Conditions <sup>1</sup>	B (B) 14.8 (15.9)	B (B) 12.0 (11.8)	C (C) 20.3 (20.1)	B (B) 19.1 (19.4)	D (D) 52.9 (52.8)	C (C) 22.4 (22.2)		
	B – 12.2 (B – 12.2)		C – 20.0 (B – 19.9)						
	B – 12.2 (B – 12.2)		C – 20.0 (B – 19.9)						
Saturday Midday Peak Hour	Year 2022 Existing Conditions	A 8.1	A 8.4	B 15.5	B 14.0	D 49.3	B 17.9		
	A – 8.4		B – 15.2						
	Year 2028 Total Conditions <sup>1</sup>	B (B) 12.2 (12.9)	B (B) 10.5 (10.7)	B (B) 19.4 (19.7)	B (B) 18.8 (19.5)	D (D) 47.3 (47.4)	C (C) 21.1 (21.5)		
	B – 10.7 (B – 10.9)		B – 19.2 (B – 19.7)						

Letter denotes Level of Service; Delay is measured in seconds.

L – Left Turn; T – Through; R – Right Turn

1. The operation of the intersection assuming the Year 2028 traffic volumes **without** the potential 7,000 square-foot restaurant is shown without parentheses.

The operation of the intersection assuming the Year 2028 traffic volumes **with** the potential 7,000 square-foot restaurant is shown in parentheses.

Table 5

CAPACITY ANALYSIS RESULTS – TWO-WAY STOP SIGN CONTROL  
 EAST LOOP ROAD WITH JAHNS DRIVE AND RICE LAKE SQUARE NORTHERN ACCESS DRIVE

Intersection	Weekday Morning Peak Hour		Weekday Evening Peak Hour		Saturday Midday Peak Hour	
	LOS	Delay	LOS	Delay	LOS	Delay
<b>Year 2022 Existing Peak Hour Conditions</b>						
• Northbound Left Turn	B	12.3	C	17.2	B	14.3
• Northbound Through/Right Turn	B	10.1	B	12.7	B	12.9
• Southbound Left Turn/Through	B	12.8	C	15.6	B	13.4
• Southbound Right Turn	A	9.6	B	10.9	B	10.0
• Westbound Left Turn	A	7.7	A	8.0	A	0.0
• Eastbound Left Turn	A	7.7	A	8.3	A	7.9
<b>Year 2028 Total Peak Hour Conditions<sup>1</sup></b>						
• Northbound Left Turn	B (B)	13.3 (13.5)	C (C)	20.1 (20.7)	C (C)	15.7 (16.1)
• Northbound Through/Right Turn	B (B)	10.4 (10.5)	B (B)	13.5 (13.7)	B (B)	13.5 (13.7)
• Southbound Left Turn/Through	B (B)	13.8(14.1)	C (C)	16.8 (17.1)	B (B)	14.2 (14.5)
• Southbound Right Turn	A (A)	9.7(9.8)	B (B)	11.5 (11.6)	B (B)	10.2 (10.3)
• Westbound Left Turn	A (A)	7.8 (7.8)	A (A)	8.1 (8.1)	A (A)	0.0 (0.0)
• Eastbound Left Turn	A (A)	7.8 (7.8)	A (A)	8.4 (8.5)	A (A)	8.0 (8.0)
LOS = Level of Service						
Delay is measured in seconds.						
1. The operation of the intersection assuming the Year 2028 traffic volumes <b>without</b> the potential 7,000 square-foot restaurant is shown without parentheses.						
The operation of the intersection assuming the Year 2028 traffic volumes <b>with</b> the potential 7,000 square-foot restaurant is shown in parentheses.						

Table 6

CAPACITY ANALYSIS RESULTS – TWO-WAY STOP SIGN CONTROL  
 EAST LOOP ROAD WITH CHICK-FIL-A ACCESS DRIVE AND RICE LAKE SQUARE SOUTHERN ACCESS DRIVE

Intersection	Weekday Morning Peak Hour		Weekday Evening Peak Hour		Saturday Midday Peak Hour	
	LOS	Delay	LOS	Delay	LOS	Delay
<b>Year 2022 Existing Peak Hour Conditions</b>						
• Eastbound Left Turn	B	12.1	C	16.7	C	15.7
• Eastbound Right Turn	B	10.1	B	11.8	B	11.4
• Westbound Left Turn	B	13.0	C	21.5	C	23.2
• Westbound Through/Right Turn	A	9.7	B	12.0	B	11.5
• Northbound Left Turn	A	7.7	A	7.9	A	7.8
• Southbound Left Turn	A	7.9	A	8.4	A	8.1
<b>Year 2028 Total Peak Hour Conditions<sup>1</sup></b>						
• Eastbound Left Turn	B (B)	13.4 (14.1)	C (C)	21.5 (22.3)	C (C)	19.7 (21.7)
• Eastbound Right Turn	B (B)	10.4 (10.4)	B (B)	13.6 (13.8)	B (B)	12.7 (12.9)
• Westbound Left Turn	C (C)	15.0(16.3)	E (F)	48.8 (65.7)	E (F)	49.2 (77.2)
• Westbound Through/Right Turn	B (B)	10.3 (10.5)	B (B)	13.2 (13.5)	B (B)	12.4 (12.7)
• Northbound Left Turn	A (A)	7.8 (7.8)	A (A)	8.2 (8.2)	A (A)	8.1 (8.1)
• Southbound Left Turn	A (A)	8.1 (8.2)	A (A)	8.6 (8.8)	A (A)	8.3 (8.5)
LOS = Level of Service Delay is measured in seconds.						
1. The operation of the intersection assuming the Year 2028 traffic volumes <b>without</b> the potential 7,000 square-foot restaurant is shown without parentheses. The operation of the intersection assuming the Year 2028 traffic volumes <b>with</b> the potential 7,000 square-foot restaurant is shown in parentheses.						

Table 7

CAPACITY ANALYSIS RESULTS – TWO-WAY STOP SIGN CONTROL  
 EAST LOOP ROAD WITH ILLINOIS INSTITUTE OF TECHNOLOGY ACCESS DRIVE AND  
 RICE LAKE SQUARE MIDDLE ACCESS DRIVE

Intersection	Weekday Morning Peak Hour		Weekday Evening Peak Hour		Saturday Midday Peak Hour	
	LOS	Delay	LOS	Delay	LOS	Delay
<b>Year 2022 Existing Peak Hour Conditions</b>						
• Eastbound Left Turn	B	11.8	B	13.0	B	12.1
• Eastbound Right Turn	A	9.8	B	10.1	A	9.7
• Westbound Approach	A	0.0	B	11.7	A	0.0
• Northbound Left Turn	A	0.0	A	7.9	A	7.8
• Southbound Left Turn	A	0.0	A	8.0	A	0.0
<b>Year 2028 Total Peak Hour Conditions<sup>1</sup></b>						
• Eastbound Left Turn	B (B)	12.8 (13.0)	B (B)	14.2 (14.4)	B (B)	13.2 (13.4)
• Eastbound Right Turn	A (B)	9.9 (10.0)	B (B)	10.3 (10.4)	A (B)	9.9 (10.0)
• Westbound Approach	B (B)	12.3 (12.5)	B (C)	14.8 (15.0)	B (B)	12.9 (13.2)
• Northbound Left Turn	A (A)	7.9 (7.9)	A (A)	8.0 (8.0)	A (A)	7.8 (7.9)
• Southbound Left Turn	A (A)	8.0 (8.0)	A (A)	8.1 (8.1)	A (A)	8.0 (8.0)
LOS = Level of Service Delay is measured in seconds.						
1. The operation of the intersection assuming the Year 2028 traffic volumes <b>without</b> the potential 7,000 square-foot restaurant is shown without parentheses. The operation of the intersection assuming the Year 2028 traffic volumes <b>with</b> the potential 7,000 square-foot restaurant is shown in parentheses.						

## Discussion and Recommendations

The following summarizes how the intersections are projected to operate and identifies any roadway and traffic control improvements necessary to accommodate the development-generated traffic.

### *East Loop Road with IL 56*

The following summarizes the results of the capacity analyses at this intersection:

- The intersection currently operates overall at Level of Service (LOS) B during the weekday morning, weekday evening, and Saturday midday peak hours. Assuming the Year 2028 total conditions with and without the potential 7,000 square-foot restaurant, this intersection is projected to continue to operate at LOS B during the weekday morning peak hour and at LOS C during the weekday evening and Saturday midday peak hours.
- All the IL 56 movements currently operate at LOS A or B during the weekday morning, weekday evening, and Saturday midday peak hours. Assuming the Year 2028 total conditions with and without the potential 7,000 square-foot restaurant, all the IL 56 movements are projected to operate on the threshold of LOS B/C or better during all three peak hours.
- The southbound approach of East Loop Road currently operates at LOS D during the weekday evening and Saturday midday peak hours and on the threshold between LOS D/E during the weekday morning peak hour. The lower level of service along the East Loop Road approach is due to the longer traffic signal cycle at this intersection and the fact that IL 56 is the primary road at this intersection and receives most of the green time. Assuming the Year 2028 total conditions with and without the potential 7,000 square-foot restaurant, the East Loop Road approach is projected to continue to operate at the same levels of service during all three peak hours. Further, the 95<sup>th</sup> percentile queues along the southbound approach of East Loop Road are not projected to extend to or past the Chick-fil-A access drive/Rice Lake Square southern access drive assuming the Year 2028 traffic conditions with and without the potential 7,000 square-foot restaurant during the weekday morning and Saturday afternoon peak hours. It should be noted that the 95<sup>th</sup> percentile queue during the weekday evening peak hour is projected to extend to approximately the Chick-fil-A access drive/Rice Lake Square southern access drive and, as is the case under existing conditions, may extend past the Chick-fil-A access drive/Rice Lake Square southern access drive on occasion during the weekday evening peak period.

As such, this intersection has sufficient reserve capacity to accommodate the Year 2028 total traffic volumes with and without the potential 7,000 square-foot restaurant and no roadway improvements or traffic control modifications are required. The operation of this intersection is enhanced due in part to the fact that (1) it is a T-intersection, which operates more efficiently than a four-way intersection, and (2) dual left-turn lanes are provided along the southbound approach of East Loop Road.

### *East Loop Road with Chick-fil-A Access Drive and the Rice Lake Square Southern Access Drive*

The results of the capacity analysis indicate that all critical movements at this intersection currently operate at LOS C or better during the weekday morning, weekday evening, and Saturday midday peak hours.

Assuming the Year 2028 total conditions with and without the potential 7,000 square-foot restaurant, all the critical movements are projected to continue operating at LOS C or better during all the peak hours except for the westbound left-turn movement from the Chick-fil-A access drive. The westbound left-turn movement is projected to operate at LOS B during the weekday morning peak hour and LOS E during the weekday evening and Saturday midday peak hours assuming the Year 2028 total conditions without the potential 7,000 square-foot restaurant. Assuming the potential 7,000 square-foot restaurant, the westbound left-turn movement is projected to operate at LOS F during the weekday evening and Saturday midday peak hours. 95<sup>th</sup> percentile queues along the Chick-fil-A access drive are projected to be three to four vehicles during the weekday evening and Saturday midday peak hours without the 7,000 square-foot restaurant, which is consistent with the observations of the existing operations. With the 7,000 square-foot restaurant, the 95<sup>th</sup> percentile queues along the Chick-fil-A access drive are projected to increase to five to six vehicles during the weekday evening and Saturday midday peak hours. As can be expected and discussed above, the delays and the queues along the access drive are projected to increase with the addition of the potential 7,000 square-foot restaurant. It is important to note that any additional delay experienced along the Chick-fil-A access drive will only impact traffic exiting the access drive and will not impact the through traffic along East Loop Road. As such, this intersection generally has sufficient reserve capacity to accommodate the Year 2028 total traffic volumes and no roadway improvements or traffic control modifications are required.

### *East Loop Road with IIT Access Drive and the Rice Lake Square Middle Access Drive*

The results of the capacity analysis indicate that all critical movements/approaches at this intersection currently operate at LOS B or better during the weekday morning, weekday evening, and Saturday midday peak hours. Assuming Year 2028 total conditions with and without the potential 7,000 square-foot restaurant, all critical movements are projected to operate on the threshold between LOS B/C or better during the peak hours. As such, this intersection has sufficient reserve capacity to accommodate the Year 2028 total traffic volumes and no roadway improvements or traffic control modifications are required.

### *East Loop Road with Jahns Drive and the Rice Lake Square Northern Access Drive*

The results of the capacity analysis indicate that all critical movements at this intersection currently operate at LOS C or better during the weekday morning, weekday evening, and Saturday midday peak hours. Assuming Year 2028 total conditions with and without the potential 7,000 square-foot restaurant, all the critical movements are projected to continue operating at LOS C or better during all three peak hours. As such, this intersection has sufficient reserve capacity to accommodate the Year 2028 total traffic volumes and no roadway improvements or traffic control modifications are required.

## Gap Study

Gap studies are conducted to determine whether a roadway traffic stream has sufficient available gaps to enable traffic to enter or exit a particular road or access drive. This is accomplished by surveying the number and lengths of available gaps in the traffic stream of a roadway. Gaps are defined as the headways (spacing) of the traffic flow (vehicles) on the roadway with the right-of-way at unsignalized intersections. The distribution of available gaps in a traffic stream is dependent on the traffic volume, the directional distribution of the traffic, the number of lanes on the roadway, and the degree and type of platooning in the traffic flow. (It should be noted that vehicles typically do not travel at constant [equal] headways. Vehicles tend to travel in platoons or groups, with varying headways [gaps] between successive vehicles.) The demand for gaps in the traffic flow is a function of the number of vehicles entering and exiting a road or access drive.

As part of the traffic study, a gap study was performed at the intersection of East Loop Road with the Chick-fil-A access drive and the Rice Lake Square southern access drive to determine if sufficient gaps are available to accommodate the additional traffic projected to be using the two access drives. The gap study was conducted in December 2022 during the weekday morning, weekday evening, and Saturday midday peak hours. **Table 8** shows the number of available gaps in the East Loop Road traffic stream and the additional traffic estimated to be generated to and from the access drives without the potential 7,000 square-foot restaurant. It is important to note the following concerning the gap study:

- If a vehicle was observed turning to or from either of the access drives, those gaps were not assumed to be available for the additional traffic projected to use the two access drives.
- The gap study did not include follow-up gaps, which consist of the shorter gaps required for any additional vehicles stacked behind the first vehicle waiting at the access drive.

As such, the gap study provides a conservative (worst-case) analysis. Based on the results of the gap study, adequate gaps are available in the East Loop Road traffic stream to accommodate the additional traffic projected to use the two access drives.

Table 8  
 RESULTS OF EAST LOOP ROAD GAP STUDY AT  
 CHICK-FIL-A ACCESS DRIVE/RICE LAKE SQUARE SOUTHERN ACCESS DRIVE

	Total Gaps	Demand For Gaps
<b>Northbound Gaps (Southbound Left-Turn and Westbound Right-Turn Movements)</b>		
• Weekday Morning Peak Hour	114	2
• Weekday Evening Peak Hour	183	8
• Saturday Midday Peak Hour	120	4
<b>Southbound Gaps (Northbound Left-Turn and Eastbound Right-Turn Movements)</b>		
• Weekday Morning Peak Hour	77	15
• Weekday Evening Peak Hour	144	24
• Saturday Midday Peak Hour	106	53
<b>Combined Gaps (Eastbound Left-Turn and Westbound Left-Turn Movements)</b>		
• Weekday Morning Peak Hour	318	13
• Weekday Evening Peak Hour	167	37
• Saturday Midday Peak Hour	210	22
Notes:		
<ul style="list-style-type: none"> <li>• Gap study only surveyed the available gaps at the intersection and did not include gaps currently used by existing traffic or follow-up gaps.</li> <li>• Demand for gaps represents the additional traffic estimated to be generated to and from the access drives.</li> </ul>		

## Potential Modifications and/or Improvements

The traffic study has shown that the East Loop Road corridor is currently and projected to continue generally operating well and functioning typical of a commercial collector road. Nevertheless, per the request of the City of Wheaton, the traffic study also examined the following additional modifications and/or improvements that could potentially enhance the operation of the corridor.

### Relocation of the Chick-fil-A Access Drive

Access spacing and access management is an important concept of roadway design as it (1) minimizes turning conflicts between access drives and (2) allows for sufficient distance between access drives in order to provide appropriate-sized turn lanes along the road serving the access drives. Relocating the Chick-fil-A access drive further north will compromise the excellent access spacing and management that is currently designed along East Loop Road, particularly the north-south section, due to the following:

- The relocation of the access drive will result in three access drives within approximately 400 feet of each other. As a result, the relocated Chick-fil-A access drive will be located only approximately 200 feet from both the Rice Lake Square middle access drive/IIT access drive and the Rice Lake Square southern access drive, creating off-set intersections with both access drives. Generally, four-legged intersections are preferred to off-set T-intersections.
- If the relocated access drive is located approximately halfway between the two Rice Lake Square access drives, only approximately 160 to 170 feet will be available to accommodate back-to-back left-turn lanes on East Loop Road serving the relocated Chick-fil-A access drive and the Rice Lake Square middle access drive which may require a center, left-turn lane instead of back-to-back turn lanes. As such, the back-to-back separate left-turn lanes or the center, two-way left-turn lane will substantially reduce the left-turn stacking and taper, which will likely impact the flow of traffic along East Loop Road.

Further, the majority of the Chick-fil-A access drive traffic is traveling to and from the south on East Loop Road. As such, the relocation of the access drive will result in additional travel for the majority of the traffic traveling to and from this access drive.

### Additional Access Drive Serving the Chick-fil-A and Car Wash Sites

Given the discussion above concerning access spacing and management, an additional access drive serving the Chick-fil-A and car wash sites would likely be restricted to right-turn in and right-turn out movements only. Based on the following, an additional right-turn in/right-turn out access drive will likely provide limited mitigation to the operation of the existing Chick-fil-A access drive:

- The amount of traffic that will use the access drive will be limited as it will likely be restricted to right-turn movements only, particularly inbound right-turn movements.
- The through/right-turn lane at the existing Chick-fil-A access drive is currently and projected to continue operating at a good level of service with limited delay.

- The additional access drive would provide limited mitigation to the left-turn movement from the Chick-fil-A access drive to southbound East Loop Road, which is the critical movement along this access drive.

### Installation of a Traffic Signal

In order to determine if a traffic signal is warranted at intersection of East Loop Road with the Chick-fil-A access drive and the Rice Lake Square southern access drive, the warrants that control the installation of traffic signals were compared to the Year 2028 total traffic volumes. The results of the review showed that the Year 2028 traffic volumes do not meet Warrant 3 (Peak Hour Volumes). It should be noted that the weekday evening and Saturday midday peak hour volumes either meet or are close to meeting Warrant 1 (Eight Hour Volumes) and Warrant 2 (Four Hour Volumes) for one hour of the day. However, the minimum volumes must be met for eight hours in a day to meet Warrant 1 or four hours in a day to meet Warrant 2. While the traffic counts for this study were only performed for a limited number of hours, it does not appear that the projected traffic volumes would meet Warrant 1 or Warrant 2. More importantly, a traffic signal is not appropriate at this intersection given its proximity (approximately 580 feet) to the signalized intersection of IL 56 with East Loop Road. Further, a traffic signal is not needed at this intersection as the traffic study has shown that (1) the critical movements at this intersection currently and are projected to continue to generally operate at good levels of service with limited delay and queuing and (2) the East Loop Road traffic stream has sufficient gaps to accommodate the traffic entering and exiting the two access drives.

In addition, a review of the warrants that control the installation of traffic signals and the Year 2028 total traffic volumes shows that a traffic signal is not warranted at the intersection of East Loop Road with Jahns Drive and the Rice Lake Square northern access drive.

### Widening of East Loop Road at its Intersection with IL 56

The capacity and operation of the southbound approach of East Loop Road can be enhanced by adding a third southbound lane along East Loop Road in order to provide dual left-turn lanes and a separate right-turn lane. A review of alternative capacity analyses performed at this intersection shows that the addition of the third lane would reduce the delay along the southbound approach of East Loop Road by two to three seconds and reduce the queuing along the southbound approach of East Loop Road by approximately 20 percent (20 to 55 feet). However, as the traffic study has shown, the southbound approach of East Loop Road at its intersection with IL 56 is currently and projected to continue operating at Level of Service D or on the threshold of LOS D/E. All vehicles generally clear the intersection on each green phase and are projected to continue to do so assuming the Year 2028 total traffic volumes. It should be noted that the additional lane will reduce the queuing along the southbound approach of East Loop Road. As discussed previously, assuming the Year 2028 traffic volumes and the existing lane geometrics, the 95<sup>th</sup> percentile queues are projected to extend to approximately the Chick-fil-A access drive/Rice Lake Square southern access drive during the evening peak period and, as is the case under existing conditions, may extend past the Chick-fil-A access drive/Rice Lake Square southern access drive on occasion during the weekday evening peak period.

## Prohibiting Right Turns on Red from Westbound IL 56 to Northbound East Loop Road

Currently, the IL 56 westbound separate right-turn lane at its signalized intersection with East Loop Road does not have an overlap (green arrow) phase with the southbound East Loop Road phase. As such, the westbound right-turn movement has a stop condition (red ball) during the (1) southbound East Loop Road phase and (2) IL 56 eastbound left-turn phase. However, right turns on red are permitted from the IL 56 westbound right-turn lane. While many right-turn movements occur when they have the stopped condition, the results of the gap study and capacity analyses show that the East Loop Road traffic stream has sufficient gaps to accommodate the traffic to and from the access drives along East Loop Road.

It should be noted that prohibiting right turns on red will limit when traffic is able to flow along northbound East Loop Road and provide additional gaps along East Loop Road. However, restricting right turn on red movements will also result in additional delay for the IL 56 westbound right-turn movements and longer queues along the right-turn lane. In addition, IDOT will need to approve the restriction of right turns on red at this location.

## 6. Conclusion

Based on the preceding analyses and recommendations, the following conclusions have been made:

- Field observations and the results of the capacity analyses reveal the following concerning the existing operation of the East Loop Road corridor:
  - Overall, the corridor generally operates well with limited congestion. Some delay and queueing occur along the corridor. However, any queueing generally dissipates quickly and is typical of collector roads serving commercial areas.
  - The East Loop Road southbound approach at its intersection with IL 56 operates well and all vehicles typically clear the intersection during each green phase. Further, the southbound queues rarely extend past the Rice Lake Square southern access drive and the Chick-fil-A access drive. The few occasions that the queue extended past the two access drives, the queue quickly dissipated.
  - Jahns Road and all the access drives in the study area typically operate well. The Chick-fil-A access drive and Rice Lake Square southern access drive have an average queue of one to two vehicles with a typical peak queue of three to four vehicles. However, it is important to note that the queues generally dissipated quickly.
- A review of the crash data over the past five years shows that the East Loop Road corridor is experiencing a low incidence of crashes.
- The traffic volumes in the study area were projected to Year 2028 which include (1) a three percent increase in the existing traffic volumes to account for ambient growth, (2) the traffic to be generated by the Wash U car wash, and (3) the traffic to be generated by the vacant space at the Rice Lake Square shopping center.
- Assuming the Year 2028 traffic conditions with and without the potential 7,000 square-foot restaurant, the IL 56/East Loop Road intersection and all the critical movements at the stop sign controlled intersections are projected to operate at LOS D or better. It should be noted that the left-turn movement from the Chick-fil-A access drive is projected to operate at LOS E or F and the East Loop Road southbound approach at IL 56 is projected to operate on the threshold between LOS D/E during certain peak periods.

- The results of the gap study conducted along East Loop Road showed that sufficient gaps are available in the East Loop Road traffic stream to accommodate the traffic turning to and from the Chick-fil-A access drive and the Rice Lake Square southern access drive.
- In general, the existing roadway system has sufficient reserve capacity to accommodate the Year 2028 projected traffic volumes with and without the potential 7,000 square-foot restaurant.

The traffic study also examined the following additional modifications and/or improvements that could potentially enhance the operation of the corridor:

- Relocating the Chick-fil-A access drive further north will compromise the excellent access spacing and management that is currently designed along East Loop Road, particularly the north-south section, as it would result in (1) three access drives within approximately 400 feet and (2) would significantly reduce the left-turn lanes/stacking along East Loop Road.
- In order to maintain appropriate access spacing and management, an additional access drive serving the Chick-fil-A and car wash sites would likely be restricted to right-turn in and right-turn out movements only which will provide limited mitigation to the operation of the existing Chick-fil-A access drive.
- A traffic signal is not required at the East Loop Road/Chick-fil-A access drive/Rice Lake Square southern access drive intersection and is not appropriate at this intersection given its proximity (approximately 580 feet) to the signalized intersection of IL 56 with East Loop Road.
- Widening the southbound approach of East Loop Road to provide a separate right-turn lane at its intersection with IL 56 will increase the capacity of this approach. However, the capacity analyses have shown that the additional lane will provide only minimal reductions in the average delay experienced at this intersection. However, it will reduce the queueing and the potential for the queue to block the Chick-fil-A access drive/Rice Lake Square southern access drive.
- Prohibiting right turns on red from westbound IL 56 to northbound East Loop Road will limit when traffic is able to flow along northbound East Loop Road and provide additional gaps along East Loop Road. However, restricting right turn on red movements will also result in additional delay for the IL 56 westbound right-turn movements and longer queues along the right-turn lane. In addition, IDOT will need to approve the restriction of right turns on red at this location.

# Appendix

Traffic Count Summary Sheets  
CMAP 2050 Projections Letter  
Figure A  
Level of Service Criteria  
Capacity Analysis Summary Sheets

# Traffic Count Summary Sheets

Type of peak hour being reported: User-Defined

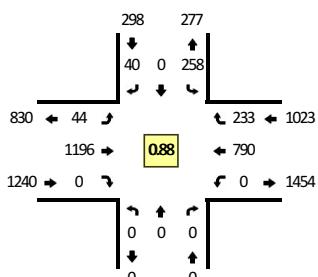
### Method for determining peak hour: Total Entering Volume

**LOCATION:** E Loop Rd -- IL 56

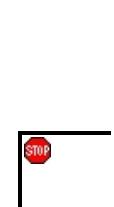
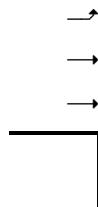
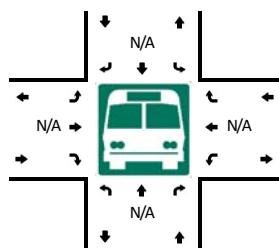
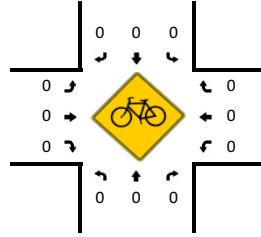
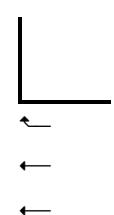
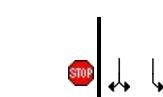
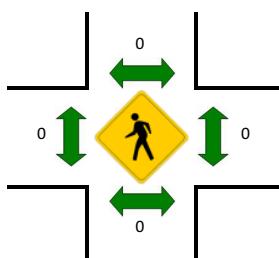
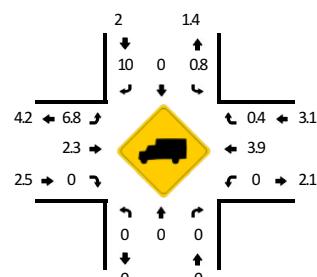
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QC JOB #: 16029801

**DATE:** Tue, Dec 6 2022



Peak-Hour: 7:30 AM -- 8:30 AM  
Peak 15-Min: 7:45 AM -- 8:00 AM



### *Comments:*

Report generated on 12/15/2022 11:09 AM

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Type of peak hour being reported: User-Defined

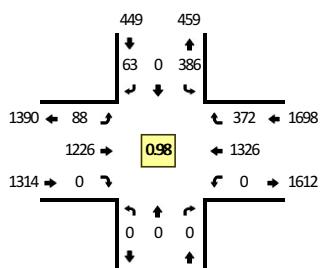
Method for determining peak hour: Total Entering Volume

LOCATION: E Loop Rd -- IL 56

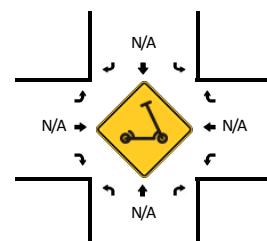
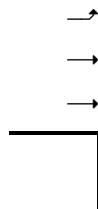
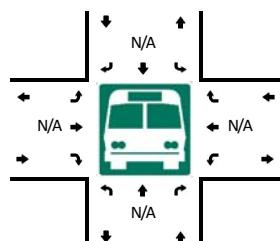
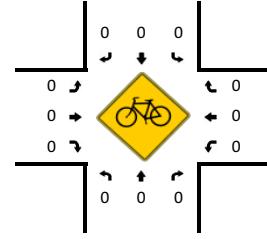
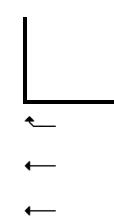
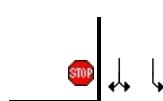
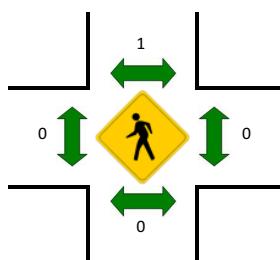
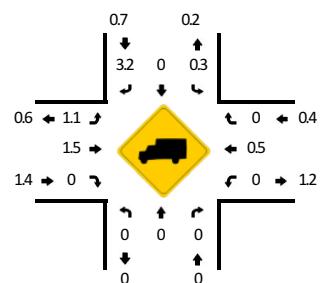
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CITY/STATE: Wheaton, IL

DATE: Tue, Dec 6 2022



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Peak 15-Min: 4:30 PM -- 4:45 PM



15-Min Count Period Beginning At	E Loop Rd (Northbound)				E Loop Rd (Southbound)				IL 56 (Eastbound)				IL 56 (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
4:00 PM	0	0	0	0	97	0	10	0	18	261	0	0	0	368	78	0	832	
4:15 PM	0	0	0	0	70	0	10	0	24	291	0	0	0	353	92	0	840	
4:30 PM	0	0	0	0	93	0	19	0	20	308	0	1	0	349	89	0	879	
4:45 PM	0	0	0	0	94	0	16	0	14	303	0	0	0	328	99	0	854	3405
5:00 PM	0	0	0	0	97	0	17	0	23	317	0	0	0	323	82	0	859	3432
5:15 PM	0	0	0	0	102	0	11	0	30	298	0	0	0	326	102	0	869	3461
5:30 PM	0	0	0	0	108	0	17	0	18	284	0	0	0	312	106	0	845	3427
5:45 PM	0	0	0	0	80	0	25	0	31	236	0	0	0	311	104	0	787	3360
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	0	0	0	0	372	0	76	0	80	1232	0	4	0	1396	356	0	3516	
Heavy Trucks	0	0	0	0	4	0	4	0	0	16	0	0	0	4	0	0	28	
Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
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Comments:

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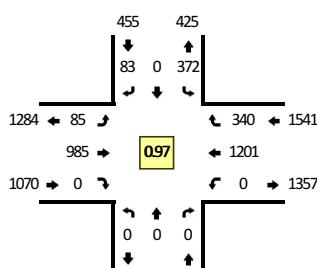
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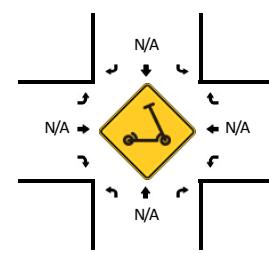
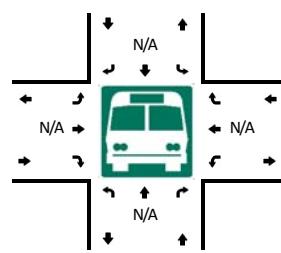
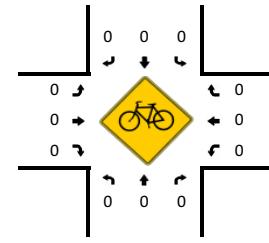
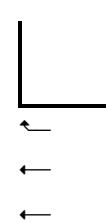
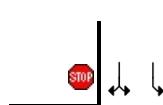
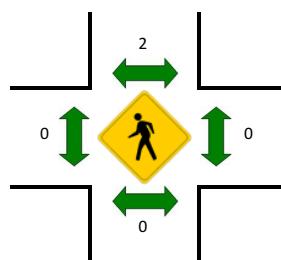
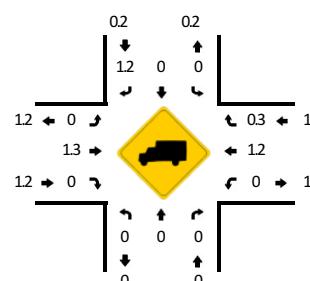
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**DATE:** Sat, Dec 10 2022



**Peak-Hour: 12:30 PM -- 1:30 PM**  
**Peak 15-Min: 1:15 PM -- 1:30 PM**



15-Min Count Period Beginning At	E Loop Rd (Northbound)				E Loop Rd (Southbound)				IL 56 (Eastbound)				IL 56 (Westbound)				Total	Hourly Totals	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U			
11:30 AM	0	0	0	0	72	0	10	0	17	226	0	0	0	285	73	0	683	2910	
11:45 AM	0	0	0	0	83	0	18	0	15	204	0	0	0	298	92	0	710		
12:00 PM	0	0	0	0	97	0	19	0	19	274	0	0	0	251	90	0	750		
12:15 PM	0	0	0	0	105	0	17	0	19	248	0	0	0	290	88	0	767		
12:30 PM	0	0	0	0	79	0	23	0	26	235	0	0	0	311	84	0	758		
12:45 PM	0	0	0	0	93	0	23	0	11	270	0	0	0	294	95	0	786		
1:00 PM	0	0	0	0	90	0	14	0	26	248	0	0	0	276	79	0	733		
1:15 PM	0	0	0	0	110	0	23	0	22	232	0	0	0	320	82	0	789		
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total		
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U			
All Vehicles	0	0	0	0	440	0	92	0	88	928	0	0	0	1280	328	0	3156	48	
Heavy Trucks	0	0	0	0	0	0	0	0	0	16	0	0	0	28	4	0			
Buses																			
Pedestrians																			
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0				
Scooters																			

### *Comments:*

Report generated on 12/15/2022 11:10 AM

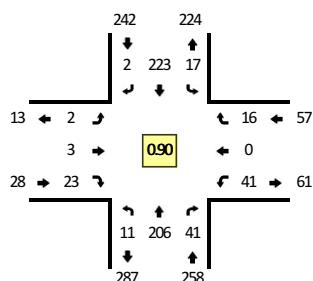
SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

Type of peak hour being reported: User-Defined

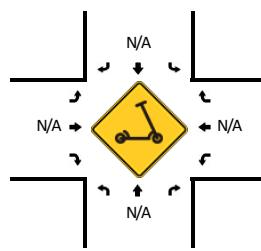
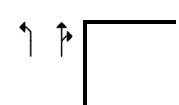
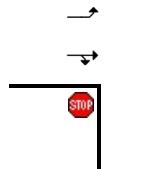
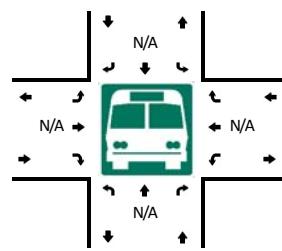
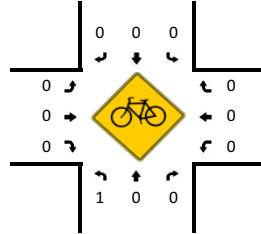
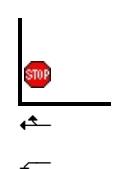
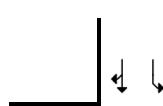
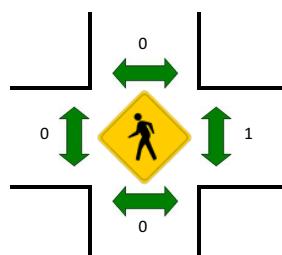
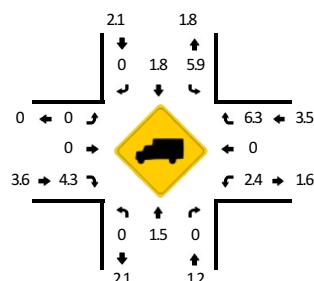
Method for determining peak hour: Total Entering Volume

**LOCATION:** E Loop Rd -- Rice Lake Square Southern Dwy/Chick-Fil-A Access Dwy  
**CITY/STATE:** Wheaton, IL

**QC JOB #:** 16029804  
**DATE:** Tue, Dec 6 2022



**Peak-Hour: 7:30 AM -- 8:30 AM**  
**Peak 15-Min: 7:30 AM -- 7:45 AM**



15-Min Count Period Beginning At	E Loop Rd (Northbound)				E Loop Rd (Southbound)				Rice Lake Square Southern Dwy/Chick-Fil-A Access Dwy (Eastbound)				Rice Lake Square Southern Dwy/Chick-Fil-A Access Dwy (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
7:00 AM	0	16	3	0	1	46	0	0	0	0	5	0	1	0	1	0	73	
7:15 AM	1	37	7	0	3	59	0	0	0	0	2	0	4	0	0	0	113	
7:30 AM	1	56	6	0	7	72	1	0	0	2	7	0	7	0	4	0	163	
7:45 AM	1	53	14	0	5	64	0	0	2	0	8	0	7	0	6	0	160	509
8:00 AM	2	50	9	0	3	44	1	0	0	1	4	0	12	0	4	0	130	566
8:15 AM	7	47	12	0	2	43	0	0	0	0	4	0	15	0	2	0	132	585
8:30 AM	0	45	6	0	5	46	1	0	0	1	6	0	6	0	5	0	121	543
8:45 AM	3	46	10	0	8	33	0	0	1	0	9	0	6	2	7	0	125	508
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound					Total
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	4	224	24	0	28	288	4	0	0	8	28	0	28	0	16	0	652	
Heavy Trucks	0	4	0		4	4	0		0	0	0		0	0	0		12	
Buses																		
Pedestrians		0				0				0				0				0
Bicycles	4	0	0		0	0	0		0	0	0		0	0	0		4	
Scooters																		

Comments:

Report generated on 12/15/2022 11:09 AM

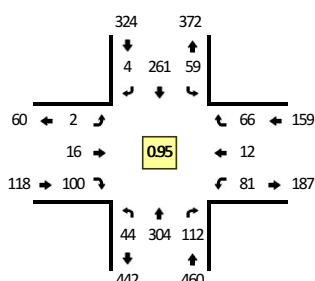
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Type of peak hour being reported: User-Defined

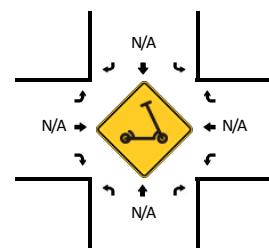
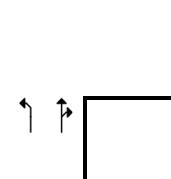
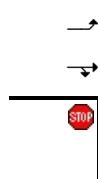
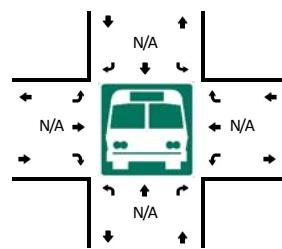
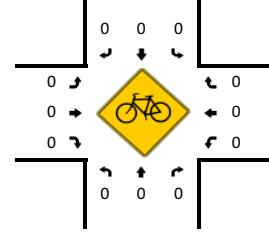
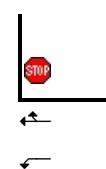
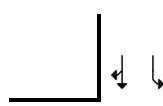
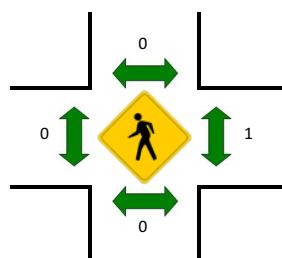
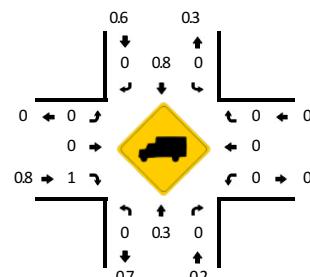
Method for determining peak hour: Total Entering Volume

**LOCATION:** E Loop Rd -- Rice Lake Square Southern Dwy/Chick-Fil-A Access Dwy  
**CITY/STATE:** Wheaton, IL

**QC JOB #:** 16029805  
**DATE:** Tue, Dec 6 2022



**Peak-Hour: 4:30 PM -- 5:30 PM**  
**Peak 15-Min: 5:15 PM -- 5:30 PM**



15-Min Count Period Beginning At	E Loop Rd (Northbound)				E Loop Rd (Southbound)				Rice Lake Square Southern Dwy/Chick-Fil-A Access Dwy (Eastbound)				Rice Lake Square Southern Dwy/Chick-Fil-A Access Dwy (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
4:00 PM	12	67	16	0	6	65	5	1	1	2	27	0	15	2	2	0	221	
4:15 PM	14	68	32	0	18	51	2	0	2	2	16	0	17	2	7	0	231	
4:30 PM	8	75	23	0	23	69	0	0	1	4	29	0	16	6	10	0	264	
4:45 PM	16	79	22	0	15	50	1	0	1	3	32	0	21	1	22	0	263	979
5:00 PM	7	72	23	0	12	70	2	0	0	6	21	0	19	1	21	0	254	1012
5:15 PM	13	78	44	0	9	72	1	0	0	3	18	0	25	4	13	0	280	1061
5:30 PM	9	76	31	0	16	73	4	0	1	1	28	0	28	1	24	0	292	1089
5:45 PM	18	82	35	0	13	42	1	0	0	2	23	0	32	3	14	0	265	1091
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound					
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Total	
All Vehicles	52	312	176	0	36	288	4	0	0	12	72	0	100	16	52	0	1120	
Heavy Trucks	0	4	0		0	4	0		0	0	0		0	0	0		8	
Buses																		
Pedestrians	0				0				0				0				0	
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0	
Scooters																		

*Comments:*

Report generated on 12/15/2022 11:10 AM

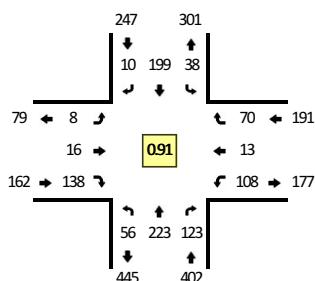
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Type of peak hour being reported: User-Defined

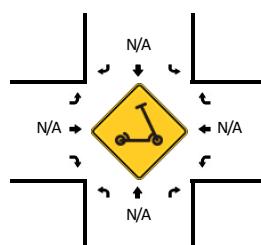
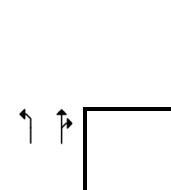
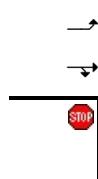
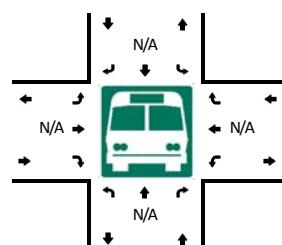
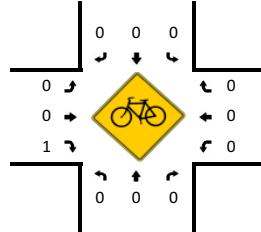
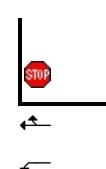
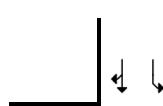
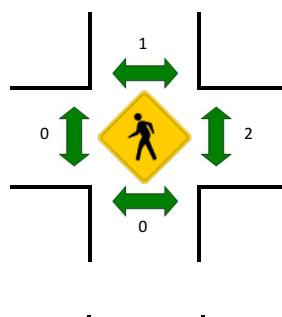
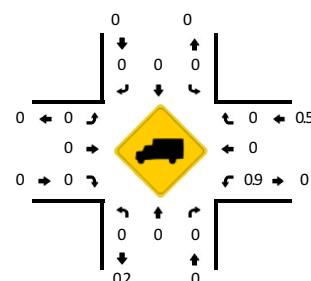
Method for determining peak hour: Total Entering Volume

**LOCATION:** E Loop Rd -- Rice Lake Square Southern Dwy/Chick-Fil-A Access Dwy  
**CITY/STATE:** Wheaton, IL

**QC JOB #:** 16029806  
**DATE:** Sat, Dec 10 2022



Peak-Hour: 12:30 PM -- 1:30 PM  
Peak 15-Min: 12:45 PM -- 1:00 PM



15-Min Count Period Beginning At	E Loop Rd (Northbound)				E Loop Rd (Southbound)				Rice Lake Square Southern Dwy/Chick-Fil-A Access Dwy (Eastbound)				Rice Lake Square Southern Dwy/Chick-Fil-A Access Dwy (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
11:30 AM	14	47	24	0	11	36	2	0	0	6	33	0	21	0	14	0	208	
11:45 AM	21	65	22	0	17	51	0	0	1	6	30	0	18	3	15	0	249	
12:00 PM	21	61	20	0	20	60	2	0	1	6	37	0	23	0	17	0	268	
12:15 PM	14	58	34	0	20	48	2	0	2	4	43	0	28	3	14	0	270	995
12:30 PM	12	51	32	0	8	53	4	0	2	8	28	0	23	4	21	0	246	1033
12:45 PM	14	73	26	0	10	45	1	0	3	2	40	0	34	5	23	0	276	1060
1:00 PM	11	56	31	0	9	47	2	0	1	2	34	0	26	2	14	0	235	1027
1:15 PM	19	43	34	0	11	54	3	0	2	4	36	0	25	2	12	0	245	1002
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound					
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Total	
All Vehicles	56	292	104	0	40	180	4	0	12	8	160	0	136	20	92	0	1104	
Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	
Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Scooters	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

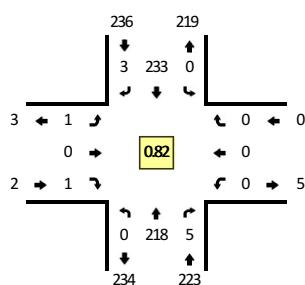
Comments:

Report generated on 12/15/2022 11:10 AM

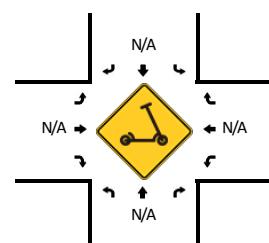
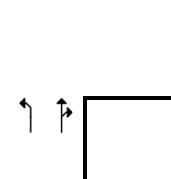
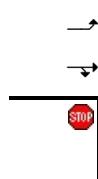
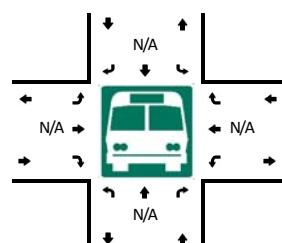
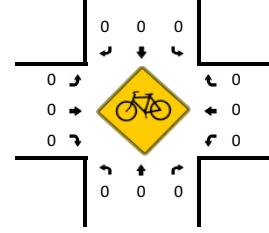
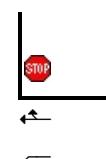
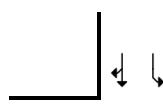
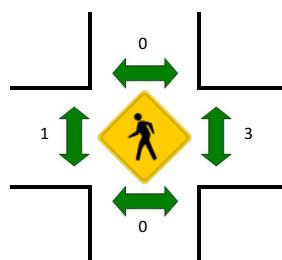
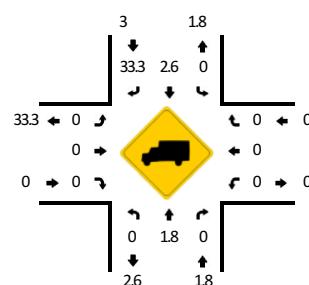
SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

**LOCATION:** E Loop Rd -- Rice Lake Square Middle Dwy/IIT Rice Campus Dwy  
**CITY/STATE:** Wheaton, IL

**QC JOB #:** 16029807  
**DATE:** Tue, Dec 6 2022



**Peak-Hour: 7:30 AM -- 8:30 AM**  
**Peak 15-Min: 7:30 AM -- 7:45 AM**



15-Min Count Period Beginning At	E Loop Rd (Northbound)				E Loop Rd (Southbound)				Rice Lake Square Middle Dwy/IIT Rice Campus Dwy (Eastbound)				Rice Lake Square Middle Dwy/IIT Rice Campus Dwy (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
7:00 AM	0	20	0	0	0	49	0	0	0	0	1	0	0	0	0	0	70	
7:15 AM	1	38	0	0	0	72	0	0	0	0	1	0	1	0	0	0	113	
7:30 AM	0	60	2	0	0	76	1	0	0	0	1	0	0	0	0	0	140	
7:45 AM	0	58	1	0	0	67	1	0	0	0	0	0	0	0	0	0	127	450
8:00 AM	0	56	1	0	0	48	1	0	0	0	0	0	0	0	0	0	106	486
8:15 AM	0	44	1	0	0	42	0	0	1	0	0	0	0	0	0	0	88	461
8:30 AM	0	50	0	0	1	54	1	0	0	0	1	0	0	0	0	0	107	428
8:45 AM	1	52	0	0	0	39	0	0	0	0	0	0	0	0	0	0	92	393
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound					
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Total	
All Vehicles	0	240	8	0	0	304	4	0	0	0	4	0	0	0	0	0	560	
Heavy Trucks	0	4	0		0	12	0		0	0	0		0	0	0	0	16	
Buses																		
Pedestrians																		
Bicycles																		
Scooters																		

*Comments:*

Report generated on 12/15/2022 11:09 AM

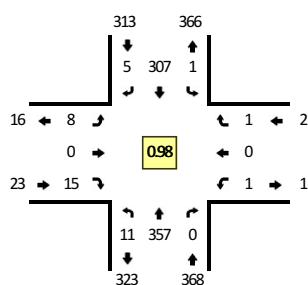
SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

Type of peak hour being reported: User-Defined

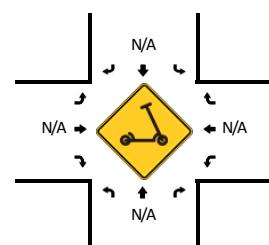
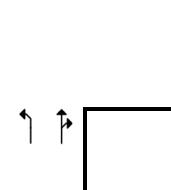
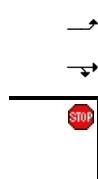
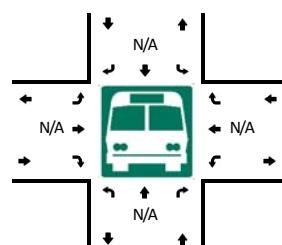
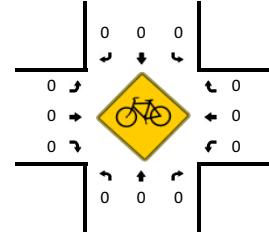
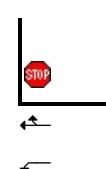
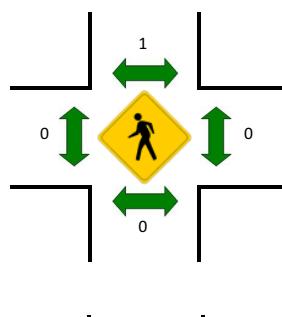
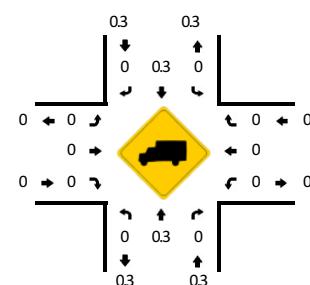
Method for determining peak hour: Total Entering Volume

**LOCATION:** E Loop Rd -- Rice Lake Square Middle Dwy/IIT Rice Campus Dwy  
**CITY/STATE:** Wheaton, IL

**QC JOB #:** 16029808  
**DATE:** Tue, Dec 6 2022



**Peak-Hour: 4:30 PM -- 5:30 PM**  
**Peak 15-Min: 4:45 PM -- 5:00 PM**



15-Min Count Period Beginning At	E Loop Rd (Northbound)				E Loop Rd (Southbound)				Rice Lake Square Middle Dwy/IIT Rice Campus Dwy (Eastbound)				Rice Lake Square Middle Dwy/IIT Rice Campus Dwy (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
4:00 PM	4	64	0	0	0	72	1	0	0	0	5	0	0	0	0	0	146	
4:15 PM	3	76	1	0	0	75	2	0	3	0	1	0	1	0	0	0	162	
4:30 PM	4	81	0	0	0	78	1	0	2	0	9	0	0	0	0	0	175	
4:45 PM	1	102	0	0	0	73	1	0	3	0	0	0	0	0	0	0	180	663
5:00 PM	3	89	0	0	0	75	2	0	2	0	2	0	1	0	0	0	174	691
5:15 PM	3	85	0	0	1	81	1	0	1	0	4	0	0	0	1	0	177	706
5:30 PM	0	105	0	0	0	85	4	0	0	0	4	0	0	0	0	0	198	729
5:45 PM	4	89	0	0	0	59	0	0	1	0	1	0	0	0	0	0	154	703
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound					Total
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	4	408	0	0	0	292	4	0	12	0	0	0	0	0	0	0	720	
Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Buses																		
Pedestrians																		
Bicycles																		
Scooters																		

Comments:

Report generated on 12/15/2022 11:10 AM

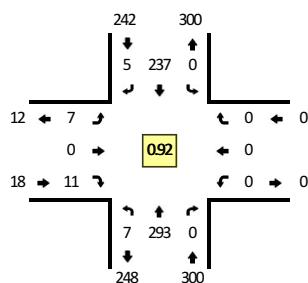
SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

Type of peak hour being reported: User-Defined

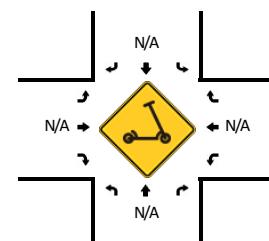
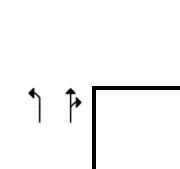
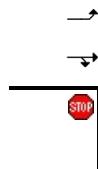
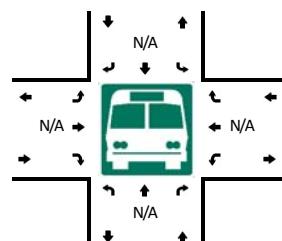
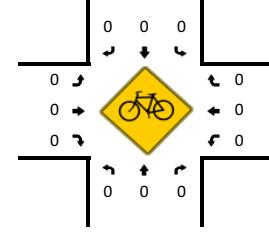
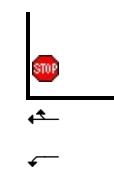
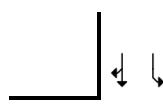
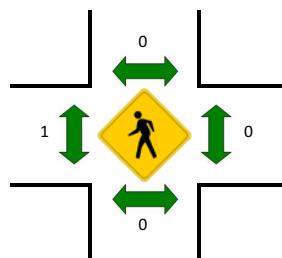
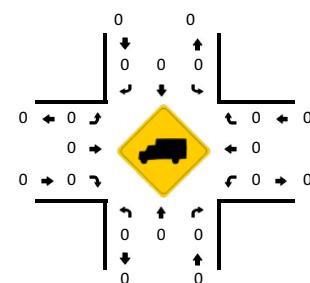
Method for determining peak hour: Total Entering Volume

**LOCATION:** E Loop Rd -- Rice Lake Square Middle Dwy/IIT Rice Campus Dwy  
**CITY/STATE:** Wheaton, IL

**QC JOB #:** 16029809  
**DATE:** Sat, Dec 10 2022



**Peak-Hour: 12:30 PM -- 1:30 PM**  
**Peak 15-Min: 12:45 PM -- 1:00 PM**



15-Min Count Period Beginning At	E Loop Rd (Northbound)				E Loop Rd (Southbound)				Rice Lake Square Middle Dwy/IIT Rice Campus Dwy (Eastbound)				Rice Lake Square Middle Dwy/IIT Rice Campus Dwy (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
11:30 AM	1	58	0	0	0	44	0	0	0	0	5	0	0	0	0	0	108	
11:45 AM	4	77	0	0	1	64	3	0	1	0	3	0	1	0	0	0	154	
12:00 PM	2	76	0	0	0	79	3	0	0	1	2	0	0	1	0	0	164	
12:15 PM	0	72	0	0	0	71	5	0	0	0	1	0	0	0	0	0	149	
12:30 PM	1	76	0	0	0	61	1	0	2	0	4	0	0	0	0	0	145	612
12:45 PM	4	90	0	0	0	52	2	0	1	0	4	0	0	0	0	0	153	611
1:00 PM	1	72	0	0	0	58	1	0	3	0	0	0	0	0	0	0	135	582
1:15 PM	1	55	0	0	0	66	1	0	1	0	3	0	0	0	0	0	127	560
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound					Total
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	16	360	0	0	0	208	8	0	4	0	16	0	0	0	0	0	612	
Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Scooters	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

*Comments:*

Report generated on 12/15/2022 11:10 AM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212



Kenig Lindgren O'Hara Aboona, Inc.  
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018  
(847)518-9990 kpachowicz@kloainc.com

Count Name: Loop Rd with Jahns Dr TMC  
Site Code:  
Start Date: 06/02/2022  
Page No: 1

### Turning Movement Data

Start Time	Loop Rd Eastbound						Loop Rd Westbound						Jahns Dr Northbound						Jahns Dr Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:00 AM	0	5	15	4	0	24	0	1	32	3	0	36	0	4	1	0	0	5	0	13	1	12	0	26	91
7:15 AM	0	5	43	5	0	53	0	3	40	1	0	44	0	1	0	0	1	1	0	12	1	10	0	23	121
7:30 AM	0	0	39	2	0	41	0	4	43	6	0	53	0	4	1	0	0	5	0	16	1	11	0	28	127
7:45 AM	0	6	36	8	0	50	0	9	42	8	0	59	0	4	2	7	0	13	0	16	2	12	0	30	152
Hourly Total	0	16	133	19	0	168	0	17	157	18	0	192	0	13	4	7	1	24	0	57	5	45	0	107	491
8:00 AM	0	3	39	7	0	49	0	6	38	1	0	45	0	9	2	7	0	18	0	12	2	5	0	19	131
8:15 AM	0	5	37	4	1	46	0	2	49	5	0	56	0	3	0	2	0	5	0	12	1	9	3	22	129
8:30 AM	0	7	36	3	0	46	0	1	44	5	0	50	0	5	2	4	0	11	0	12	1	6	0	19	126
8:45 AM	0	3	34	10	0	47	0	2	60	2	1	64	0	4	0	2	1	6	0	9	7	15	0	31	148
Hourly Total	0	18	146	24	1	188	0	11	191	13	1	215	0	21	4	15	1	40	0	45	11	35	3	91	534
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
4:00 PM	0	13	53	14	5	80	0	0	69	10	0	79	0	13	0	3	0	16	0	11	2	8	0	21	196
4:15 PM	0	7	51	9	2	67	0	0	67	14	0	81	0	10	1	2	0	13	0	5	2	5	0	12	173
4:30 PM	0	10	57	5	0	72	0	0	68	11	0	79	0	13	3	5	0	21	0	8	1	5	0	14	186
4:45 PM	0	13	59	11	1	83	0	1	77	17	1	95	0	13	4	2	0	19	0	5	1	9	0	15	212
Hourly Total	0	43	220	39	8	302	0	1	281	52	1	334	0	49	8	12	0	69	0	29	6	27	0	62	767
5:00 PM	0	11	74	13	1	98	0	0	68	21	1	89	0	20	6	10	0	36	0	6	3	11	0	20	243
5:15 PM	0	9	79	19	0	107	0	0	87	28	0	115	0	12	3	3	0	18	0	5	1	16	0	22	262
5:30 PM	0	10	71	20	1	101	0	1	80	17	0	98	0	12	2	4	0	18	0	12	0	8	0	20	237
5:45 PM	0	17	51	14	0	82	0	0	71	16	1	87	0	12	1	2	0	15	0	8	1	10	0	19	203
Hourly Total	0	47	275	66	2	388	0	1	306	82	2	389	0	56	12	19	0	87	0	31	5	45	0	81	945
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
11:00 AM	0	10	54	12	2	76	0	1	47	8	2	56	0	15	4	0	0	19	0	9	1	11	0	21	172
11:15 AM	0	14	54	10	0	78	0	1	55	12	0	68	0	17	1	1	2	19	0	5	0	9	1	14	179
11:30 AM	0	10	63	14	1	87	0	0	67	11	0	78	0	5	1	0	0	6	0	8	4	16	0	28	199
11:45 AM	0	14	72	18	0	104	0	0	64	14	0	78	0	19	3	0	0	22	0	11	2	11	0	24	228
Hourly Total	0	48	243	54	3	345	0	2	233	45	2	280	0	56	9	1	2	66	0	33	7	47	1	87	778
12:00 PM	0	15	50	18	1	83	0	0	64	4	0	68	0	16	4	1	0	21	0	5	2	12	0	19	191
12:15 PM	0	8	52	18	0	78	0	2	77	20	0	99	0	15	5	2	0	22	0	19	2	13	0	34	233
12:30 PM	0	6	56	16	0	78	0	0	78	5	0	83	0	14	5	1	0	20	0	11	2	12	0	25	206
12:45 PM	0	8	72	15	0	95	0	0	59	8	0	67	0	18	5	0	0	23	0	6	4	5	0	15	200
Hourly Total	0	37	230	67	1	334	0	2	278	37	0	317	0	63	19	4	0	86	0	41	10	42	0	93	830
1:00 PM	0	15	55	14	0	84	0	0	66	10	0	76	0	25	4	0	0	29	0	6	3	8	0	17	206
1:15 PM	0	8	66	16	0	90	0	0	62	7	0	69	0	14	5	1	0	20	0	9	4	8	0	21	200
1:30 PM	0	8	55	8	0	71	0	1	77	13	1	91	0	12	3	1	0	16	0	5	1	12	0	18	196

1:45 PM	0	7	49	9	0	65	0	0	65	19	0	84	0	13	2	0	0	15	0	4	3	12	0	19	183	
Hourly Total	0	38	225	47	0	310	0	1	270	49	1	320	0	64	14	2	0	80	0	24	11	40	0	75	785	
Grand Total	0	247	1472	316	15	2035	0	35	1716	296	7	2047	0	322	70	60	4	452	0	260	55	281	4	596	5130	
Approach %	0.0	12.1	72.3	15.5	-	-	0.0	1.7	83.8	14.5	-	-	0.0	71.2	15.5	13.3	-	-	0.0	43.6	9.2	47.1	-	-	-	
Total %	0.0	4.8	28.7	6.2	-	39.7	0.0	0.7	33.5	5.8	-	39.9	0.0	6.3	1.4	1.2	-	8.8	0.0	5.1	1.1	5.5	-	11.6	-	
Lights	0	246	1451	313	-	2010	0	33	1695	292	-	2020	0	321	67	59	-	447	0	255	51	274	-	580	5057	
% Lights	-	99.6	98.6	99.1	-	98.8	-	94.3	98.8	98.6	-	98.7	-	99.7	95.7	98.3	-	98.9	-	98.1	92.7	97.5	-	97.3	98.6	
Buses	0	0	10	0	-	10	0	0	9	1	-	10	0	0	0	0	-	0	0	1	0	4	-	5	25	
% Buses	-	0.0	0.7	0.0	-	0.5	-	0.0	0.5	0.3	-	0.5	-	0.0	0.0	0.0	-	0.0	-	0.4	0.0	1.4	-	0.8	0.5	
Single-Unit Trucks	0	0	8	2	-	10	0	2	9	1	-	12	0	0	1	1	-	2	0	1	0	0	-	1	25	
% Single-Unit Trucks	-	0.0	0.5	0.6	-	0.5	-	5.7	0.5	0.3	-	0.6	-	0.0	1.4	1.7	-	0.4	-	0.4	0.0	0.0	-	0.2	0.5	
Articulated Trucks	0	0	0	1	-	1	0	0	1	1	-	2	0	0	0	0	-	0	0	0	0	1	-	1	4	
% Articulated Trucks	-	0.0	0.0	0.3	-	0.0	-	0.0	0.1	0.3	-	0.1	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.4	-	0.2	0.1	
Bicycles on Road	0	1	3	0	-	4	0	0	2	1	-	3	0	1	2	0	-	3	0	3	4	2	-	9	19	
% Bicycles on Road	-	0.4	0.2	0.0	-	0.2	-	0.0	0.1	0.3	-	0.1	-	0.3	2.9	0.0	-	0.7	-	1.2	7.3	0.7	-	1.5	0.4	
Pedestrians	-	-	-	-	-	15	-	-	-	-	-	7	-	-	-	-	-	4	-	-	-	-	4	-	-	
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Kenig Lindgren O'Hara Aboona, Inc.  
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018  
(847)518-9990 kpachowicz@kloainc.com

Count Name: Loop Rd with Jahns Dr TMC  
Site Code:  
Start Date: 06/02/2022  
Page No: 3

### Turning Movement Peak Hour Data (7:30 AM)

Start Time	Loop Rd Eastbound						Loop Rd Westbound						Jahns Dr Northbound						Jahns Dr Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:30 AM	0	0	39	2	0	41	0	4	43	6	0	53	0	4	1	0	0	5	0	16	1	11	0	28	127
7:45 AM	0	6	36	8	0	50	0	9	42	8	0	59	0	4	2	7	0	13	0	16	2	12	0	30	152
8:00 AM	0	3	39	7	0	49	0	6	38	1	0	45	0	9	2	7	0	18	0	12	2	5	0	19	131
8:15 AM	0	5	37	4	1	46	0	2	49	5	0	56	0	3	0	2	0	5	0	12	1	9	3	22	129
Total	0	14	151	21	1	186	0	21	172	20	0	213	0	20	5	16	0	41	0	56	6	37	3	99	539
Approach %	0.0	7.5	81.2	11.3	-	-	0.0	9.9	80.8	9.4	-	-	0.0	48.8	12.2	39.0	-	-	0.0	56.6	6.1	37.4	-	-	-
Total %	0.0	2.6	28.0	3.9	-	34.5	0.0	3.9	31.9	3.7	-	39.5	0.0	3.7	0.9	3.0	-	7.6	0.0	10.4	1.1	6.9	-	18.4	-
PHF	0.000	0.583	0.968	0.656	-	0.930	0.000	0.583	0.878	0.625	-	0.903	0.000	0.556	0.625	0.571	-	0.569	0.000	0.875	0.750	0.771	-	0.825	0.887
Lights	0	13	145	19	-	177	0	21	170	20	-	211	0	20	5	16	-	41	0	56	5	35	-	96	525
% Lights	-	92.9	96.0	90.5	-	95.2	-	100.0	98.8	100.0	-	99.1	-	100.0	100.0	100.0	-	100.0	-	100.0	83.3	94.6	-	97.0	97.4
Buses	0	0	4	0	-	4	0	0	2	0	-	2	0	0	0	0	-	0	0	0	0	1	-	1	7
% Buses	-	0.0	2.6	0.0	-	2.2	-	0.0	1.2	0.0	-	0.9	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	2.7	-	1.0	1.3
Single-Unit Trucks	0	0	2	1	-	3	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	3
% Single-Unit Trucks	-	0.0	1.3	4.8	-	1.6	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.6
Articulated Trucks	0	0	0	1	-	1	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	1
% Articulated Trucks	-	0.0	0.0	4.8	-	0.5	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.2
Bicycles on Road	0	1	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	1	-	2	3
% Bicycles on Road	-	7.1	0.0	0.0	-	0.5	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	16.7	2.7	-	2.0	0.6
Pedestrians	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	3	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-



Kenig Lindgren O'Hara Aboona, Inc.  
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018  
(847)518-9990 kpachowicz@kloainc.com

Count Name: Loop Rd with Jahns Dr TMC  
Site Code:  
Start Date: 06/02/2022  
Page No: 4

## Turning Movement Peak Hour Data (4:30 PM)



Kenig Lindgren O'Hara Aboona, Inc.  
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018  
(847)518-9990 kpachowicz@kloainc.com

Count Name: Loop Rd with Jahns Dr TMC  
Site Code:  
Start Date: 06/02/2022  
Page No: 5

## Turning Movement Peak Hour Data (12:30 PM)

# CMAP 2050 Projections Letter



# Chicago Metropolitan Agency for Planning

433 West Van Buren Street  
Suite 450  
Chicago, IL 60607

312-454-0400  
cmap.illinois.gov

January 6, 2023

Kelly Pachowicz  
Consultant  
Kenig, Lindgren, O'Hara, Aboona, Inc.  
9575 West Higgins Road  
Suite 400  
Rosemont, IL 60018

**Subject: Butterfield Road @ East Loop Drive**  
IDOT

Dear Ms. Pachowicz:

In response to a request made on your behalf and dated January 5, 2023, we have developed year 2050 average daily traffic (ADT) projections for the subject location.

ROAD SEGMENT	Current ADT (2019)	Year 2050 ADT
Butterfield Rd @ East Loop Dr	27,100	32,300

Traffic projections are developed using existing ADT data provided in the request letter and the results from the October 2022 CMAP Travel Demand Analysis. The regional travel model uses CMAP 2050 socioeconomic projections and assumes the implementation of the ON TO 2050 Comprehensive Regional Plan for the Northeastern Illinois area. The provision of this data in support of your request does not constitute a CMAP endorsement of the proposed development or any subsequent developments.

If you have any questions, please call me at (312) 386-8806.

Sincerely,

A handwritten signature in black ink, appearing to read "Jose Rodriguez".

Jose Rodriguez, PTP, AICP  
Senior Planner, Research & Analysis

cc: Rios (IDOT)  
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Figure A



# Level of Service Criteria

## LEVEL OF SERVICE CRITERIA

Signalized Intersections		
Level of Service	Interpretation	Average Control Delay (seconds per vehicle)
A	Favorable progression. Most vehicles arrive during the green indication and travel through the intersection without stopping.	≤10
B	Good progression, with more vehicles stopping than for Level of Service A.	>10 - 20
C	Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear. Number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.	>20 - 35
D	The volume-to-capacity ratio is high and either progression is ineffective or the cycle length is too long. Many vehicles stop and individual cycle failures are noticeable.	>35 - 55
E	Progression is unfavorable. The volume-to-capacity ratio is high and the cycle length is long. Individual cycle failures are frequent.	>55 - 80
F	The volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.	>80.0
Unsignalized Intersections		
Level of Service	Average Total Delay (SEC/VEH)	
A	0 - 10	
B	> 10 - 15	
C	> 15 - 25	
D	> 25 - 35	
E	> 35 - 50	
F	> 50	

Source: *Highway Capacity Manual*, 2010.

**Capacity Analysis Summary Sheets**  
Existing Weekday Morning Peak Hour

Lanes, Volumes, Timings  
1: Butterfield Road & E Loop Road

01/24/2023



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑	↑↑	↑↑	↑	↑↑	
Traffic Volume (vph)	44	1196	790	233	258	40
Future Volume (vph)	44	1196	790	233	258	40
Ideal Flow (vphpl)	1900	2000	2000	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	230			220	225	150
Storage Lanes	1			1	1	0
Taper Length (ft)	250				55	
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Fr <sub>t</sub>				0.850	0.980	
Flt Protected	0.950				0.958	
Satd. Flow (prot)	1687	3725	3654	1615	3386	0
Flt Permitted	0.275				0.958	
Satd. Flow (perm)	488	3725	3654	1615	3386	0
Right Turn on Red				No	No	
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		30	
Link Distance (ft)		1427	1045		481	
Travel Time (s)		21.6	15.8		10.9	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	7%	2%	4%	0%	1%	10%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	50	1359	898	265	338	0
Turn Type	pm+pt	NA	NA	Perm	Prot	
Protected Phases	5	2	6		4	
Permitted Phases	2			6		
Detector Phase	5	2	6	6	4	
Switch Phase						
Minimum Initial (s)	3.0	15.0	15.0	15.0	8.0	
Minimum Split (s)	12.5	24.5	24.5	24.5	24.0	
Total Split (s)	13.0	90.0	77.0	77.0	40.0	
Total Split (%)	10.0%	69.2%	59.2%	59.2%	30.8%	
Yellow Time (s)	3.5	4.0	4.0	4.0	4.5	
All-Red Time (s)	0.0	2.0	2.0	2.0	1.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Min	C-Min	C-Min	None	
Act Effct Green (s)	100.2	97.7	89.6	89.6	20.3	
Actuated g/C Ratio	0.77	0.75	0.69	0.69	0.16	



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.11	0.49	0.36	0.24	0.64	
Control Delay	4.7	7.3	9.6	9.2	56.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	4.7	7.3	9.6	9.2	56.9	
LOS	A	A	A	A	E	
Approach Delay		7.2	9.5		56.9	
Approach LOS		A	A		E	
Queue Length 50th (ft)	9	204	156	79	139	
Queue Length 95th (ft)	21	283	220	136	176	
Internal Link Dist (ft)		1347	965		401	
Turn Bay Length (ft)	230			220	225	
Base Capacity (vph)	463	2800	2517	1112	885	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.11	0.49	0.36	0.24	0.38	

#### Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 35 (27%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 13.9

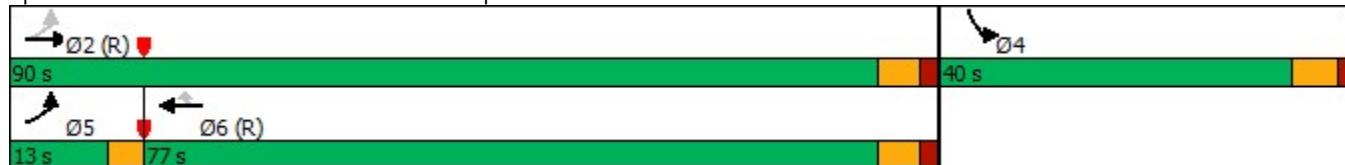
Intersection LOS: B

Intersection Capacity Utilization 50.0%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 1: Butterfield Road & E Loop Road



## Intersection

Int Delay, s/veh

2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	2	3	23	41	0	16	11	206	41	17	223	2
Future Vol, veh/h	2	3	23	41	0	16	11	206	41	17	223	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	-	0	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	0	0	4	2	0	6	0	2	0	6	2	0
Mvmt Flow	2	3	26	46	0	18	12	229	46	19	248	2

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	572	586	249	578	564	252	250	0	0	275	0	0
Stage 1	287	287	-	276	276	-	-	-	-	-	-	-
Stage 2	285	299	-	302	288	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.24	7.12	6.5	6.26	4.1	-	-	4.16	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.12	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.12	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.336	3.518	4	3.354	2.2	-	-	2.254	-	-
Pot Cap-1 Maneuver	434	425	785	427	438	777	1327	-	-	1265	-	-
Stage 1	725	678	-	730	685	-	-	-	-	-	-	-
Stage 2	727	670	-	707	677	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	416	415	785	404	427	777	1327	-	-	1265	-	-
Mov Cap-2 Maneuver	510	489	-	498	499	-	-	-	-	-	-	-
Stage 1	718	668	-	723	679	-	-	-	-	-	-	-
Stage 2	704	664	-	670	667	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	10.2	12.1			0.3			0.6				
HCM LOS	B	B										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR		
Capacity (veh/h)	1327	-	-	510	734	498	777	1265	-	-		
HCM Lane V/C Ratio	0.009	-	-	0.004	0.039	0.091	0.023	0.015	-	-		
HCM Control Delay (s)	7.7	-	-	12.1	10.1	13	9.7	7.9	-	-		
HCM Lane LOS	A	-	-	B	B	B	A	A	-	-		
HCM 95th %tile Q(veh)	0	-	-	0	0.1	0.3	0.1	0	-	-		

## Intersection

Int Delay, s/veh

0

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖
Traffic Vol, veh/h	1	0	1	0	0	0	0	219	5	0	241	3
Future Vol, veh/h	1	0	1	0	0	0	0	219	5	0	241	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	0	-	-	-	150	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	82	82	82	82	82	82	82	82	82	82	82	82
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	0	3	33
Mvmt Flow	1	0	1	0	0	0	0	267	6	0	294	4

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	566	-	296	567	568	270	298	0	0	273	0	0
Stage 1	296	-	-	270	270	-	-	-	-	-	-	-
Stage 2	270	-	-	297	298	-	-	-	-	-	-	-
Critical Hdwy	7.1	-	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	-	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	438	0	748	437	435	774	1275	-	-	1302	-	-
Stage 1	717	0	-	740	690	-	-	-	-	-	-	-
Stage 2	740	0	-	716	671	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	438	-	748	436	435	774	1275	-	-	1302	-	-
Mov Cap-2 Maneuver	532	-	-	531	509	-	-	-	-	-	-	-
Stage 1	717	-	-	740	690	-	-	-	-	-	-	-
Stage 2	740	-	-	715	671	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB			
HCM Control Delay, s	10.8	0			0			0			
HCM LOS	B	A									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR		
Capacity (veh/h)	1275	-	-	532	748	-	1302	-	-		
HCM Lane V/C Ratio	-	-	-	0.002	0.002	-	-	-	-		
HCM Control Delay (s)	0	-	-	11.8	9.8	0	0	-	-		
HCM Lane LOS	A	-	-	B	A	A	A	-	-		
HCM 95th %tile Q(veh)	0	-	-	0	0	-	0	-	-		

Intersection												
Int Delay, s/veh	3.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	14	172	21	21	179	20	20	5	16	56	6	37
Future Vol, veh/h	14	172	21	21	179	20	20	5	16	56	6	37
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	100	-	-	0	-	-	-	-	100
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	3	10	0	1	0	0	0	0	0	0	3
Mvmt Flow	16	193	24	24	201	22	22	6	18	63	7	42
Major/Minor												
Major1		Major2			Minor1		Minor2					
Conflicting Flow All	223	0	0	217	0	0	522	508	205	509	509	212
Stage 1	-	-	-	-	-	-	237	237	-	260	260	-
Stage 2	-	-	-	-	-	-	285	271	-	249	249	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.23
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.327
Pot Cap-1 Maneuver	1358	-	-	1365	-	-	468	471	841	478	470	826
Stage 1	-	-	-	-	-	-	771	713	-	749	697	-
Stage 2	-	-	-	-	-	-	727	689	-	759	704	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1358	-	-	1365	-	-	430	457	841	454	456	826
Mov Cap-2 Maneuver	-	-	-	-	-	-	515	519	-	536	517	-
Stage 1	-	-	-	-	-	-	762	704	-	740	684	-
Stage 2	-	-	-	-	-	-	672	677	-	728	696	-
Approach												
EB			WB			NE			SW			
HCM Control Delay, s	0.5		0.7			11.2			11.6			
HCM LOS	B						B					
Minor Lane/Major Mvmt		NELn1	NELn2	EBL	EBT	EBR	WBL	WBT	WBR	SWLn1	SWLn2	
Capacity (veh/h)	515	733	1358	-	-	-	1365	-	-	534	826	
HCM Lane V/C Ratio	0.044	0.032	0.012	-	-	-	0.017	-	-	0.13	0.05	
HCM Control Delay (s)	12.3	10.1	7.7	-	-	-	7.7	-	-	12.8	9.6	
HCM Lane LOS	B	B	A	-	-	-	A	-	-	B	A	
HCM 95th %tile Q(veh)	0.1	0.1	0	-	-	-	0.1	-	-	0.4	0.2	

# Capacity Analysis Summary Sheets

Existing Weekday Evening Peak Hour

Lanes, Volumes, Timings  
1: Butterfield Road & E Loop Road

01/24/2023



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑	↑↑	↑↑	↑	↑↑	
Traffic Volume (vph)	88	1226	1326	372	386	63
Future Volume (vph)	88	1226	1326	372	386	63
Ideal Flow (vphpl)	1900	2000	2000	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	230			220	225	150
Storage Lanes	1			1	1	0
Taper Length (ft)	250				55	
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Fr <sub>t</sub>				0.850	0.979	
Flt Protected	0.950				0.959	
Satd. Flow (prot)	1787	3725	3800	1615	3446	0
Flt Permitted	0.134				0.959	
Satd. Flow (perm)	252	3725	3800	1615	3446	0
Right Turn on Red				No	No	
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		30	
Link Distance (ft)		1427	1045		481	
Travel Time (s)		21.6	15.8		10.9	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	2%	0%	0%	0%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	90	1251	1353	380	458	0
Turn Type	pm+pt	NA	NA	Perm	Prot	
Protected Phases	5	2	6		4	
Permitted Phases	2			6		
Detector Phase	5	2	6	6	4	
Switch Phase						
Minimum Initial (s)	3.0	15.0	15.0	15.0	8.0	
Minimum Split (s)	12.5	24.5	24.5	24.5	24.0	
Total Split (s)	13.0	90.0	77.0	77.0	40.0	
Total Split (%)	10.0%	69.2%	59.2%	59.2%	30.8%	
Yellow Time (s)	3.5	4.0	4.0	4.0	4.5	
All-Red Time (s)	0.0	2.0	2.0	2.0	1.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Min	C-Min	C-Min	None	
Act Effct Green (s)	95.6	93.1	82.1	82.1	24.9	
Actuated g/C Ratio	0.74	0.72	0.63	0.63	0.19	



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.33	0.47	0.56	0.37	0.69	
Control Delay	8.8	9.1	15.6	13.8	54.4	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	8.8	9.1	15.6	13.8	54.4	
LOS	A	A	B	B	D	
Approach Delay		9.0	15.2		54.4	
Approach LOS		A	B		D	
Queue Length 50th (ft)	19	215	322	145	186	
Queue Length 95th (ft)	41	304	454	247	231	
Internal Link Dist (ft)		1347	965		401	
Turn Bay Length (ft)	230		220	225		
Base Capacity (vph)	297	2666	2401	1020	901	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.30	0.47	0.56	0.37	0.51	

#### Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 23 (18%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.69

Intersection Signal Delay: 17.9

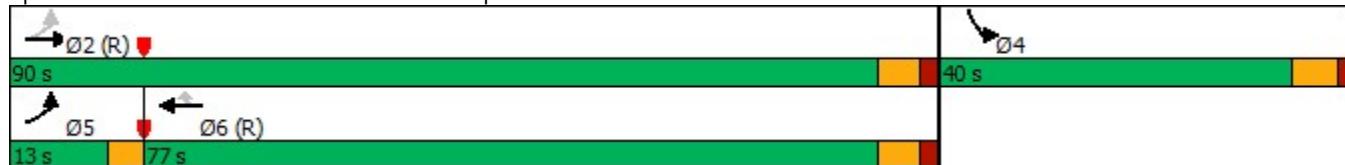
Intersection LOS: B

Intersection Capacity Utilization 66.0%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: Butterfield Road & E Loop Road



## Intersection

Int Delay, s/veh 4.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	2	16	100	81	12	66	44	304	112	59	261	4
Future Vol, veh/h	2	16	100	81	12	66	44	304	112	59	261	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	-	0	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	0	0	1	0	0	0	0	0	0	0	1	0
Mvmt Flow	2	17	105	85	13	69	46	320	118	62	275	4

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	913	931	277	933	874	379	279	0	0	438	0	0
Stage 1	401	401	-	471	471	-	-	-	-	-	-	-
Stage 2	512	530	-	462	403	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.21	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.309	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	256	269	764	248	290	672	1295	-	-	1133	-	-
Stage 1	630	604	-	577	563	-	-	-	-	-	-	-
Stage 2	548	530	-	584	603	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	208	245	764	192	264	672	1295	-	-	1133	-	-
Mov Cap-2 Maneuver	309	338	-	302	364	-	-	-	-	-	-	-
Stage 1	607	571	-	556	543	-	-	-	-	-	-	-
Stage 2	463	511	-	462	570	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB			
HCM Control Delay, s	11.9	16.8			0.8			1.5			
HCM LOS	B	C									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR	
Capacity (veh/h)	1295	-	-	309	651	302	595	1133	-	-	
HCM Lane V/C Ratio	0.036	-	-	0.007	0.188	0.282	0.138	0.055	-	-	
HCM Control Delay (s)	7.9	-	-	16.7	11.8	21.5	12	8.4	-	-	
HCM Lane LOS	A	-	-	C	B	C	B	A	-	-	
HCM 95th %tile Q(veh)	0.1	-	-	0	0.7	1.1	0.5	0.2	-	-	

## Intersection

Int Delay, s/veh 0.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖		↖		↖	↖	↖	↖	↖	↖	↖	↖
Traffic Vol, veh/h	8	0	15	1	0	1	11	361	0	1	308	5
Future Vol, veh/h	8	0	15	1	0	1	11	361	0	1	308	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	0	-	-	-	150	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	98	98	98	98	98	98	98	98	98	98	98	98
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	8	0	15	1	0	1	11	368	0	1	314	5

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	710	-	317	716	711	368	319	0	0	368	0	0
Stage 1	319	-	-	390	390	-	-	-	-	-	-	-
Stage 2	391	-	-	326	321	-	-	-	-	-	-	-
Critical Hdwy	7.1	-	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	-	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	351	0	728	348	361	682	1252	-	-	1202	-	-
Stage 1	697	0	-	638	611	-	-	-	-	-	-	-
Stage 2	637	0	-	691	655	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	348	-	728	338	357	682	1252	-	-	1202	-	-
Mov Cap-2 Maneuver	459	-	-	450	447	-	-	-	-	-	-	-
Stage 1	691	-	-	632	606	-	-	-	-	-	-	-
Stage 2	630	-	-	676	654	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	11.1	11.7			0.2			0		
HCM LOS	B	B								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1252	-	-	459	728	542	1202	-	-	
HCM Lane V/C Ratio	0.009	-	-	0.018	0.021	0.004	0.001	-	-	
HCM Control Delay (s)	7.9	-	-	13	10.1	11.7	8	-	-	
HCM Lane LOS	A	-	-	B	B	B	A	-	-	
HCM 95th %tile Q(veh)	0	-	-	0.1	0.1	0	0	-	-	

Intersection												
Int Delay, s/veh	3.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	43	270	48	1	292	77	58	16	20	24	6	41
Future Vol, veh/h	43	270	48	1	292	77	58	16	20	24	6	41
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	100	-	-	0	-	-	-	-	100
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	2
Mvmt Flow	50	314	56	1	340	90	67	19	23	28	7	48
Major/Minor												
Major1		Major2			Minor1			Minor2				
Conflicting Flow All	430	0	0	370	0	0	857	874	342	850	857	385
Stage 1	-	-	-	-	-	-	442	442	-	387	387	-
Stage 2	-	-	-	-	-	-	415	432	-	463	470	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.318
Pot Cap-1 Maneuver	1140	-	-	1200	-	-	280	290	705	283	297	663
Stage 1	-	-	-	-	-	-	598	580	-	641	613	-
Stage 2	-	-	-	-	-	-	619	586	-	583	563	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1140	-	-	1200	-	-	248	277	705	254	284	663
Mov Cap-2 Maneuver	-	-	-	-	-	-	361	375	-	371	390	-
Stage 1	-	-	-	-	-	-	572	554	-	613	612	-
Stage 2	-	-	-	-	-	-	567	585	-	521	538	-
Approach												
EB			WB			NE			SW			
HCM Control Delay, s	1			0			15.5			12.9		
HCM LOS							C			B		
Minor Lane/Major Mvmt												
Capacity (veh/h)	361	507	1140	-	-	1200	-	-	375	663		
HCM Lane V/C Ratio	0.187	0.083	0.044	-	-	0.001	-	-	0.093	0.072		
HCM Control Delay (s)	17.2	12.7	8.3	-	-	8	-	-	15.6	10.9		
HCM Lane LOS	C	B	A	-	-	A	-	-	C	B		
HCM 95th %tile Q(veh)	0.7	0.3	0.1	-	-	0	-	-	0.3	0.2		

# Capacity Analysis Summary Sheets

## Existing Saturday Midday Peak Hour

Lanes, Volumes, Timings  
1: Butterfield Road & E Loop Road

01/24/2023



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑	↑↑	↑↑	↑	↑↑	
Traffic Volume (vph)	85	985	1201	340	372	83
Future Volume (vph)	85	985	1201	340	372	83
Ideal Flow (vphpl)	1900	2000	2000	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	230			220	225	150
Storage Lanes	1			1	1	0
Taper Length (ft)	250				55	
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Fr <sub>t</sub>				0.850	0.973	
Flt Protected	0.950				0.961	
Satd. Flow (prot)	1805	3762	3762	1615	3440	0
Flt Permitted	0.155				0.961	
Satd. Flow (perm)	294	3762	3762	1615	3440	0
Right Turn on Red				No	No	
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		30	
Link Distance (ft)		1427	1045		481	
Travel Time (s)		21.6	15.8		10.9	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	1%	1%	0%	0%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	88	1015	1238	351	470	0
Turn Type	pm+pt	NA	NA	Perm	Prot	
Protected Phases	5	2	6		4	
Permitted Phases	2			6		
Detector Phase	5	2	6	6	4	
Switch Phase						
Minimum Initial (s)	3.0	15.0	15.0	15.0	8.0	
Minimum Split (s)	12.5	24.5	24.5	24.5	24.0	
Total Split (s)	13.0	80.0	67.0	67.0	40.0	
Total Split (%)	10.8%	66.7%	55.8%	55.8%	33.3%	
Yellow Time (s)	3.5	4.0	4.0	4.0	4.5	
All-Red Time (s)	0.0	2.0	2.0	2.0	1.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Min	C-Min	C-Min	None	
Act Effct Green (s)	86.4	83.9	73.1	73.1	24.1	
Actuated g/C Ratio	0.72	0.70	0.61	0.61	0.20	



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.29	0.39	0.54	0.36	0.68	
Control Delay	8.1	8.4	15.5	14.0	49.3	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	8.1	8.4	15.5	14.0	49.3	
LOS	A	A	B	B	D	
Approach Delay		8.4	15.2		49.3	
Approach LOS		A	B		D	
Queue Length 50th (ft)	18	154	276	128	174	
Queue Length 95th (ft)	40	224	397	223	218	
Internal Link Dist (ft)		1347	965		401	
Turn Bay Length (ft)	230			220	225	
Base Capacity (vph)	331	2631	2292	984	974	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.27	0.39	0.54	0.36	0.48	

#### Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 29 (24%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.68

Intersection Signal Delay: 17.9

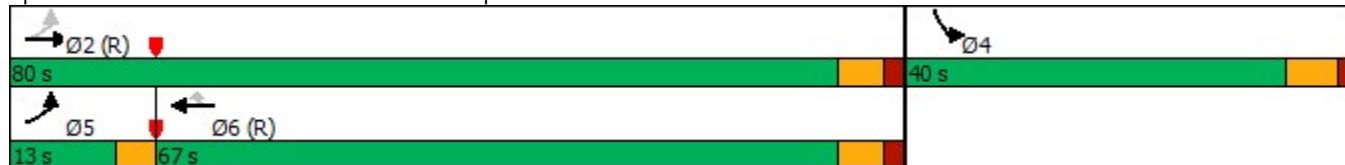
Intersection LOS: B

Intersection Capacity Utilization 62.8%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: Butterfield Road & E Loop Road



## Intersection

Int Delay, s/veh 6.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑		↑	↑	
Traffic Vol, veh/h	8	16	138	108	13	70	56	223	123	38	199	10
Future Vol, veh/h	8	16	138	108	13	70	56	223	123	38	199	10
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	-	0	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	0	1	0	0	0	0	0	0	0	0
Mvmt Flow	9	18	152	119	14	77	62	245	135	42	219	11

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	791	813	225	831	751	313	230	0	0	380	0	0
Stage 1	309	309	-	437	437	-	-	-	-	-	-	-
Stage 2	482	504	-	394	314	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.11	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.11	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.11	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.509	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	310	315	819	290	342	732	1350	-	-	1190	-	-
Stage 1	705	663	-	600	583	-	-	-	-	-	-	-
Stage 2	569	544	-	633	660	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	254	290	819	214	315	732	1350	-	-	1190	-	-
Mov Cap-2 Maneuver	345	376	-	315	402	-	-	-	-	-	-	-
Stage 1	673	640	-	572	556	-	-	-	-	-	-	-
Stage 2	473	519	-	484	637	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	11.6	18.1			1.1			1.3				
HCM LOS	B	C										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR		
Capacity (veh/h)	1350	-	-	345	730	315	649	1190	-	-		
HCM Lane V/C Ratio	0.046	-	-	0.025	0.232	0.377	0.141	0.035	-	-		
HCM Control Delay (s)	7.8	-	-	15.7	11.4	23.2	11.5	8.1	-	-		
HCM Lane LOS	A	-	-	C	B	C	B	A	-	-		
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.9	1.7	0.5	0.1	-	-		

## Intersection

Int Delay, s/veh 0.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖		↖		↖	↖	↖	↖	↖	↖	↖	↖
Traffic Vol, veh/h	7	0	11	0	0	0	7	294	0	0	236	5
Future Vol, veh/h	7	0	11	0	0	0	7	294	0	0	236	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	0	-	-	-	150	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	8	0	12	0	0	0	8	320	0	0	257	5

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	596	-	260	602	598	320	262	0	0	320	0	0
Stage 1	260	-	-	336	336	-	-	-	-	-	-	-
Stage 2	336	-	-	266	262	-	-	-	-	-	-	-
Critical Hdwy	7.1	-	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	-	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	418	0	784	414	418	725	1314	-	-	1251	-	-
Stage 1	749	0	-	682	645	-	-	-	-	-	-	-
Stage 2	682	0	-	744	695	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	416	-	784	406	415	725	1314	-	-	1251	-	-
Mov Cap-2 Maneuver	513	-	-	505	491	-	-	-	-	-	-	-
Stage 1	745	-	-	678	641	-	-	-	-	-	-	-
Stage 2	678	-	-	733	695	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	10.6	0			0.2			0		
HCM LOS	B	A								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1314	-	-	513	784	-	1251	-	-	
HCM Lane V/C Ratio	0.006	-	-	0.015	0.015	-	-	-	-	
HCM Control Delay (s)	7.8	-	-	12.1	9.7	0	0	-	-	
HCM Lane LOS	A	-	-	B	A	A	A	-	-	
HCM 95th %tile Q(veh)	0	-	-	0	0	-	0	-	-	

Intersection												
Int Delay, s/veh	3.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	37	207	61	0	271	30	71	19	2	32	13	33
Future Vol, veh/h	37	207	61	0	271	30	71	19	2	32	13	33
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	100	-	-	0	-	-	-	-	100
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	98	98	98	98	98	98	98	98	98	98	98	98
Heavy Vehicles, %	0	1	0	0	1	0	0	5	0	3	0	0
Mvmt Flow	38	211	62	0	277	31	72	19	2	33	13	34
Major/Minor	Major1		Major2		Minor1		Minor2					
Conflicting Flow All	308	0	0	273	0	0	634	626	242	622	642	293
Stage 1	-	-	-	-	-	-	318	318	-	293	293	-
Stage 2	-	-	-	-	-	-	316	308	-	329	349	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.55	6.2	7.13	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.55	-	6.13	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.55	-	6.13	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4.045	3.3	3.527	4	3.3
Pot Cap-1 Maneuver	1264	-	-	1302	-	-	395	397	802	398	395	751
Stage 1	-	-	-	-	-	-	698	648	-	713	674	-
Stage 2	-	-	-	-	-	-	699	655	-	682	637	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1264	-	-	1302	-	-	361	385	802	375	383	751
Mov Cap-2 Maneuver	-	-	-	-	-	-	459	460	-	474	468	-
Stage 1	-	-	-	-	-	-	677	629	-	692	674	-
Stage 2	-	-	-	-	-	-	655	655	-	639	618	-
Approach	EB		WB		NE		SW					
HCM Control Delay, s	1		0		14		12					
HCM LOS					B		B					
Minor Lane/Major Mvmt	NELn1		NELn2		EBL	EBT	EBR	WBL	WBT	WBR	SWLn1	SWLn2
Capacity (veh/h)	459		479		1264	-	-	1302	-	-	472	751
HCM Lane V/C Ratio	0.158		0.045		0.03	-	-	-	-	-	0.097	0.045
HCM Control Delay (s)	14.3		12.9		7.9	-	-	0	-	-	13.4	10
HCM Lane LOS	B		B		A	-	-	A	-	-	B	B
HCM 95th %tile Q(veh)	0.6		0.1		0.1	-	-	0	-	-	0.3	0.1

## Capacity Analysis Summary Sheets

Year 2028 Total Projected Weekday Morning Peak Hour  
With Restaurant



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑	↑↑	↑↑	↑	↑↑	
Traffic Volume (vph)	84	1232	814	314	304	59
Future Volume (vph)	84	1232	814	314	304	59
Ideal Flow (vphpl)	1900	2000	2000	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	230			220	225	150
Storage Lanes	1			1	1	0
Taper Length (ft)	250				55	
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Fr <sub>t</sub>				0.850	0.976	
Flt Protected	0.950				0.960	
Satd. Flow (prot)	1687	3725	3654	1615	3371	0
Flt Permitted	0.257				0.960	
Satd. Flow (perm)	456	3725	3654	1615	3371	0
Right Turn on Red				No	No	
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		30	
Link Distance (ft)		1427	1045		481	
Travel Time (s)		21.6	15.8		10.9	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	7%	2%	4%	0%	1%	10%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	95	1400	925	357	412	0
Turn Type	pm+pt	NA	NA	Perm	Prot	
Protected Phases	5	2	6		4	
Permitted Phases	2			6		
Detector Phase	5	2	6	6	4	
Switch Phase						
Minimum Initial (s)	3.0	15.0	15.0	15.0	8.0	
Minimum Split (s)	12.5	24.5	24.5	24.5	24.0	
Total Split (s)	13.0	90.0	77.0	77.0	40.0	
Total Split (%)	10.0%	69.2%	59.2%	59.2%	30.8%	
Yellow Time (s)	3.5	4.0	4.0	4.0	4.5	
All-Red Time (s)	0.0	2.0	2.0	2.0	1.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Min	C-Min	C-Min	None	
Act Effct Green (s)	97.2	94.7	83.7	83.7	23.3	
Actuated g/C Ratio	0.75	0.73	0.64	0.64	0.18	



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.23	0.52	0.39	0.34	0.68	
Control Delay	6.3	8.9	12.3	12.6	55.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	6.3	8.9	12.3	12.6	55.6	
LOS	A	A	B	B	E	
Approach Delay		8.7	12.4		55.6	
Approach LOS		A	B		E	
Queue Length 50th (ft)	19	237	180	127	170	
Queue Length 95th (ft)	39	326	256	213	207	
Internal Link Dist (ft)		1347	965		401	
Turn Bay Length (ft)	230			220	225	
Base Capacity (vph)	431	2714	2352	1039	881	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.22	0.52	0.39	0.34	0.47	

#### Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 35 (27%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.68

Intersection Signal Delay: 16.3

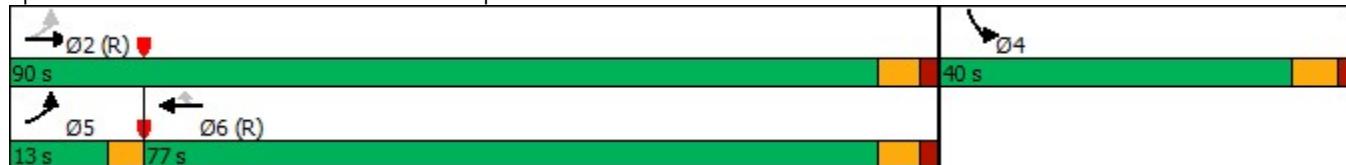
Intersection LOS: B

Intersection Capacity Utilization 52.9%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 1: Butterfield Road & E Loop Road



## Intersection

Int Delay, s/veh 3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗											
Traffic Vol, veh/h	3	3	33	71	0	29	28	273	76	32	246	4
Future Vol, veh/h	3	3	33	71	0	29	28	273	76	32	246	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	0	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	0	0	4	2	0	6	0	2	0	6	2	0
Mvmt Flow	3	3	37	79	0	32	31	303	84	36	273	4

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	770	796	275	774	756	345	277	0	0	387	0	0
Stage 1	347	347	-	407	407	-	-	-	-	-	-	-
Stage 2	423	449	-	367	349	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.24	7.12	6.5	6.26	4.1	-	-	4.16	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.12	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.12	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.336	3.518	4	3.354	2.2	-	-	2.254	-	-
Pot Cap-1 Maneuver	320	322	759	316	340	689	1298	-	-	1150	-	-
Stage 1	673	638	-	621	601	-	-	-	-	-	-	-
Stage 2	613	576	-	653	637	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	292	305	759	286	322	689	1298	-	-	1150	-	-
Mov Cap-2 Maneuver	399	397	-	398	415	-	-	-	-	-	-	-
Stage 1	657	618	-	606	587	-	-	-	-	-	-	-
Stage 2	570	562	-	599	617	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	10.7	14.6			0.6			0.9		
HCM LOS	B	B								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1298	-	-	399	705	398	689	1150	-	-
HCM Lane V/C Ratio	0.024	-	-	0.008	0.057	0.198	0.047	0.031	-	-
HCM Control Delay (s)	7.8	-	-	14.1	10.4	16.3	10.5	8.2	-	-
HCM Lane LOS	A	-	-	B	B	C	B	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0	0.2	0.7	0.1	0.1	-	-

## Intersection

Int Delay, s/veh 0.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖		↖		↖	↖	↖	↖	↖	↖	↖	↖
Traffic Vol, veh/h	2	0	5	12	0	4	6	240	59	18	265	4
Future Vol, veh/h	2	0	5	12	0	4	6	240	59	18	265	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	0	-	-	-	150	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	82	82	82	82	82	82	82	82	82	82	82	82
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	0	3	33
Mvmt Flow	2	0	6	15	0	5	7	293	72	22	323	5

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	716	-	326	716	715	329	328	0	0	365	0	0
Stage 1	370	-	-	343	343	-	-	-	-	-	-	-
Stage 2	346	-	-	373	372	-	-	-	-	-	-	-
Critical Hdwy	7.1	-	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	-	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	348	0	720	348	359	717	1243	-	-	1205	-	-
Stage 1	654	0	-	676	641	-	-	-	-	-	-	-
Stage 2	674	0	-	652	622	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	339	-	720	339	350	717	1243	-	-	1205	-	-
Mov Cap-2 Maneuver	450	-	-	451	442	-	-	-	-	-	-	-
Stage 1	650	-	-	672	637	-	-	-	-	-	-	-
Stage 2	666	-	-	635	611	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB			
HCM Control Delay, s	10.9	12.5			0.2			0.5			
HCM LOS	B	B									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR		
Capacity (veh/h)	1243	-	-	450	720	497	1205	-	-		
HCM Lane V/C Ratio	0.006	-	-	0.005	0.008	0.039	0.018	-	-		
HCM Control Delay (s)	7.9	-	-	13	10	12.5	8	-	-		
HCM Lane LOS	A	-	-	B	B	B	A	-	-		
HCM 95th %tile Q(veh)	0	-	-	0	0	0.1	0.1	-	-		

Intersection												
Int Delay, s/veh	3.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	32	215	24	21	205	20	22	5	16	56	7	41
Future Vol, veh/h	32	215	24	21	205	20	22	5	16	56	7	41
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	100	-	-	0	-	-	-	-	100
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	3	10	0	1	0	0	0	0	0	0	3
Mvmt Flow	36	242	27	24	230	22	25	6	18	63	8	46
Major/Minor												
Major1		Major2			Minor1			Minor2				
Conflicting Flow All	252	0	0	269	0	0	644	628	256	629	630	241
Stage 1	-	-	-	-	-	-	328	328	-	289	289	-
Stage 2	-	-	-	-	-	-	316	300	-	340	341	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.23
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.327
Pot Cap-1 Maneuver	1325	-	-	1306	-	-	389	402	788	398	401	795
Stage 1	-	-	-	-	-	-	689	651	-	723	677	-
Stage 2	-	-	-	-	-	-	699	669	-	679	642	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1325	-	-	1306	-	-	349	384	788	372	383	795
Mov Cap-2 Maneuver	-	-	-	-	-	-	448	461	-	468	462	-
Stage 1	-	-	-	-	-	-	670	633	-	703	665	-
Stage 2	-	-	-	-	-	-	639	657	-	640	625	-
Approach												
EB			WB			NE			SW			
HCM Control Delay, s	0.9		0.7			12			12.4			
HCM LOS	B						B					
Minor Lane/Major Mvmt		NELn1	NELn2	EBL	EBT	EBR	WBL	WBT	WBR	SWLn1	SWLn2	
Capacity (veh/h)	448	674	1325	-	-	-	1306	-	-	467	795	
HCM Lane V/C Ratio	0.055	0.035	0.027	-	-	-	0.018	-	-	0.152	0.058	
HCM Control Delay (s)	13.5	10.5	7.8	-	-	-	7.8	-	-	14.1	9.8	
HCM Lane LOS	B	B	A	-	-	-	A	-	-	B	A	
HCM 95th %tile Q(veh)	0.2	0.1	0.1	-	-	-	0.1	-	-	0.5	0.2	

## Capacity Analysis Summary Sheets

Year 2028 Total Projected Weekday Morning Peak Hour  
Without Restaurant

Lanes, Volumes, Timings  
1: Butterfield Road & E Loop Road

02/13/2023



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	76	1232	814	299	292	53
Future Volume (vph)	76	1232	814	299	292	53
Ideal Flow (vphpl)	1900	2000	2000	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	230			220	225	150
Storage Lanes	1			1	1	0
Taper Length (ft)	250				55	
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Fr <sub>t</sub>				0.850	0.977	
Flt Protected	0.950				0.959	
Satd. Flow (prot)	1687	3725	3654	1615	3373	0
Flt Permitted	0.259				0.959	
Satd. Flow (perm)	460	3725	3654	1615	3373	0
Right Turn on Red				No	No	
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		30	
Link Distance (ft)		1427	1045		481	
Travel Time (s)		21.6	15.8		10.9	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	7%	2%	4%	0%	1%	10%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	86	1400	925	340	392	0
Turn Type	pm+pt	NA	NA	Perm	Prot	
Protected Phases	5	2	6		4	
Permitted Phases	2			6		
Detector Phase	5	2	6	6	4	
Switch Phase						
Minimum Initial (s)	3.0	15.0	15.0	15.0	8.0	
Minimum Split (s)	12.5	24.5	24.5	24.5	24.0	
Total Split (s)	13.0	90.0	77.0	77.0	40.0	
Total Split (%)	10.0%	69.2%	59.2%	59.2%	30.8%	
Yellow Time (s)	3.5	4.0	4.0	4.0	4.5	
All-Red Time (s)	0.0	2.0	2.0	2.0	1.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Min	C-Min	C-Min	None	
Act Effct Green (s)	98.0	95.5	84.7	84.7	22.5	
Actuated g/C Ratio	0.75	0.73	0.65	0.65	0.17	



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.21	0.51	0.39	0.32	0.67	
Control Delay	5.9	8.5	11.8	11.8	55.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	5.9	8.5	11.8	11.8	55.9	
LOS	A	A	B	B	E	
Approach Delay		8.4	11.8		55.9	
Approach LOS		A	B		E	
Queue Length 50th (ft)	16	231	176	116	161	
Queue Length 95th (ft)	35	317	249	196	199	
Internal Link Dist (ft)		1347	965		401	
Turn Bay Length (ft)	230			220	225	
Base Capacity (vph)	436	2736	2380	1052	882	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.20	0.51	0.39	0.32	0.44	

#### Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 35 (27%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.67

Intersection Signal Delay: 15.7

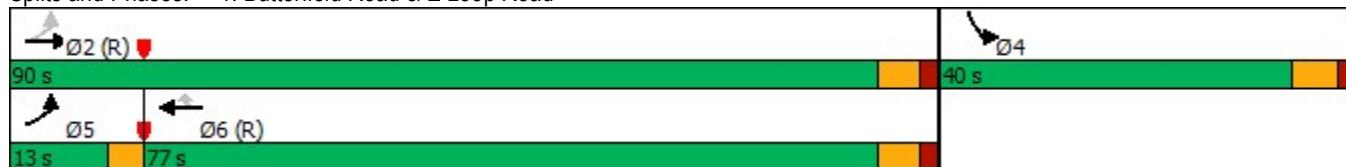
Intersection LOS: B

Intersection Capacity Utilization 52.3%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 1: Butterfield Road & E Loop Road



## Intersection

Int Delay, s/veh 2.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	3	3	33	53	0	17	28	273	53	18	246	4
Future Vol, veh/h	3	3	33	53	0	17	28	273	53	18	246	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	-	0	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	0	0	4	2	0	6	0	2	0	6	2	0
Mvmt Flow	3	3	37	59	0	19	31	303	59	20	273	4

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	719	739	275	730	712	333	277	0	0	362	0	0
Stage 1	315	315	-	395	395	-	-	-	-	-	-	-
Stage 2	404	424	-	335	317	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.24	7.12	6.5	6.26	4.1	-	-	4.16	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.12	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.12	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.336	3.518	4	3.354	2.2	-	-	2.254	-	-
Pot Cap-1 Maneuver	346	347	759	338	360	700	1298	-	-	1175	-	-
Stage 1	700	659	-	630	608	-	-	-	-	-	-	-
Stage 2	627	590	-	679	658	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	326	333	759	310	345	700	1298	-	-	1175	-	-
Mov Cap-2 Maneuver	432	423	-	419	433	-	-	-	-	-	-	-
Stage 1	683	648	-	615	593	-	-	-	-	-	-	-
Stage 2	596	576	-	632	647	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	10.6	13.9			0.6			0.5		
HCM LOS	B	B								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1298	-	-	432	712	419	700	1175	-	-
HCM Lane V/C Ratio	0.024	-	-	0.008	0.056	0.141	0.027	0.017	-	-
HCM Control Delay (s)	7.8	-	-	13.4	10.4	15	10.3	8.1	-	-
HCM Lane LOS	A	-	-	B	B	C	B	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0	0.2	0.5	0.1	0.1	-	-

## Intersection

Int Delay, s/veh 0.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖		↖		↖	↖	↖	↖	↖	↖	↖	↖
Traffic Vol, veh/h	2	0	5	12	0	4	6	228	59	18	251	4
Future Vol, veh/h	2	0	5	12	0	4	6	228	59	18	251	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	0	-	-	-	150	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	82	82	82	82	82	82	82	82	82	82	82	82
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	0	3	33
Mvmt Flow	2	0	6	15	0	5	7	278	72	22	306	5

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	684	-	309	684	683	314	311	0	0	350	0	0
Stage 1	353	-	-	328	328	-	-	-	-	-	-	-
Stage 2	331	-	-	356	355	-	-	-	-	-	-	-
Critical Hdwy	7.1	-	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	-	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	365	0	736	365	374	731	1261	-	-	1220	-	-
Stage 1	668	0	-	689	651	-	-	-	-	-	-	-
Stage 2	687	0	-	666	633	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	356	-	736	356	365	731	1261	-	-	1220	-	-
Mov Cap-2 Maneuver	464	-	-	465	454	-	-	-	-	-	-	-
Stage 1	664	-	-	685	647	-	-	-	-	-	-	-
Stage 2	679	-	-	649	622	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	10.7	12.3	0.2	0.5
HCM LOS	B	B		
<hr/>				
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1 EBLn2 WBLn1 SBL SBT SBR
Capacity (veh/h)	1261	-	-	464 736 512 1220 - -
HCM Lane V/C Ratio	0.006	-	-	0.005 0.008 0.038 0.018 - -
HCM Control Delay (s)	7.9	-	-	12.8 9.9 12.3 8 - -
HCM Lane LOS	A	-	-	B A B A - -
HCM 95th %tile Q(veh)	0	-	-	0 0 0.1 0.1 - -

Intersection												
Int Delay, s/veh	3.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	32	201	24	21	193	20	22	5	16	56	7	41
Future Vol, veh/h	32	201	24	21	193	20	22	5	16	56	7	41
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	100	-	-	0	-	-	-	-	100
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	3	10	0	1	0	0	0	0	0	0	3
Mvmt Flow	36	226	27	24	217	22	25	6	18	63	8	46
Major/Minor												
Major1		Major2			Minor1			Minor2				
Conflicting Flow All	239	0	0	253	0	0	615	599	240	600	601	228
Stage 1	-	-	-	-	-	-	312	312	-	276	276	-
Stage 2	-	-	-	-	-	-	303	287	-	324	325	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.23
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.327
Pot Cap-1 Maneuver	1340	-	-	1324	-	-	406	418	804	416	417	809
Stage 1	-	-	-	-	-	-	703	661	-	735	685	-
Stage 2	-	-	-	-	-	-	711	678	-	692	653	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1340	-	-	1324	-	-	365	399	804	389	398	809
Mov Cap-2 Maneuver	-	-	-	-	-	-	461	473	-	481	473	-
Stage 1	-	-	-	-	-	-	684	643	-	715	673	-
Stage 2	-	-	-	-	-	-	651	666	-	653	635	-
Approach												
EB			WB			NE			SW			
HCM Control Delay, s	1		0.7			11.9			12.2			
HCM LOS	B						B					
Minor Lane/Major Mvmt		NELn1	NELn2	EBL	EBT	EBR	WBL	WBT	WBR	SWLn1	SWLn2	
Capacity (veh/h)	461	689	1340	-	-	1324	-	-	480	809		
HCM Lane V/C Ratio	0.054	0.034	0.027	-	-	0.018	-	-	0.147	0.057		
HCM Control Delay (s)	13.3	10.4	7.8	-	-	7.8	-	-	13.8	9.7		
HCM Lane LOS	B	B	A	-	-	A	-	-	B	A		
HCM 95th %tile Q(veh)	0.2	0.1	0.1	-	-	0.1	-	-	0.5	0.2		

## Capacity Analysis Summary Sheets

Year 2028 Total Projected Weekday Evening Peak Hour  
With Restaurant

Lanes, Volumes, Timings  
1: Butterfield Road & E Loop Road

02/13/2023



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑	↑↑	↑↑	↑	↑↑	
Traffic Volume (vph)	129	1263	1366	469	474	115
Future Volume (vph)	129	1263	1366	469	474	115
Ideal Flow (vphpl)	1900	2000	2000	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	230			220	225	150
Storage Lanes	1			1	1	0
Taper Length (ft)	250				55	
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Fr <sub>t</sub>				0.850	0.971	
Flt Protected	0.950				0.961	
Satd. Flow (prot)	1787	3725	3800	1615	3420	0
Flt Permitted	0.109				0.961	
Satd. Flow (perm)	205	3725	3800	1615	3420	0
Right Turn on Red				No	No	
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		30	
Link Distance (ft)		1427	1045		481	
Travel Time (s)		21.6	15.8		10.9	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	2%	0%	0%	0%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	132	1289	1394	479	601	0
Turn Type	pm+pt	NA	NA	Perm	Prot	
Protected Phases	5	2	6		4	
Permitted Phases	2			6		
Detector Phase	5	2	6	6	4	
Switch Phase						
Minimum Initial (s)	3.0	15.0	15.0	15.0	8.0	
Minimum Split (s)	12.5	24.5	24.5	24.5	24.0	
Total Split (s)	13.0	90.0	77.0	77.0	40.0	
Total Split (%)	10.0%	69.2%	59.2%	59.2%	30.8%	
Yellow Time (s)	3.5	4.0	4.0	4.0	4.5	
All-Red Time (s)	0.0	2.0	2.0	2.0	1.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Min	C-Min	C-Min	None	
Act Effct Green (s)	90.3	87.8	75.8	75.8	30.2	
Actuated g/C Ratio	0.69	0.68	0.58	0.58	0.23	



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.54	0.51	0.63	0.51	0.76	
Control Delay	15.9	11.8	20.1	19.4	52.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	15.9	11.8	20.1	19.4	52.8	
LOS	B	B	C	B	D	
Approach Delay		12.2	19.9		52.8	
Approach LOS		B	B		D	
Queue Length 50th (ft)	34	268	397	234	242	
Queue Length 95th (ft)	63	343	504	353	300	
Internal Link Dist (ft)		1347	965		401	
Turn Bay Length (ft)	230			220	225	
Base Capacity (vph)	258	2514	2216	941	894	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.51	0.51	0.63	0.51	0.67	

#### Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 23 (18%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.76

Intersection Signal Delay: 22.2

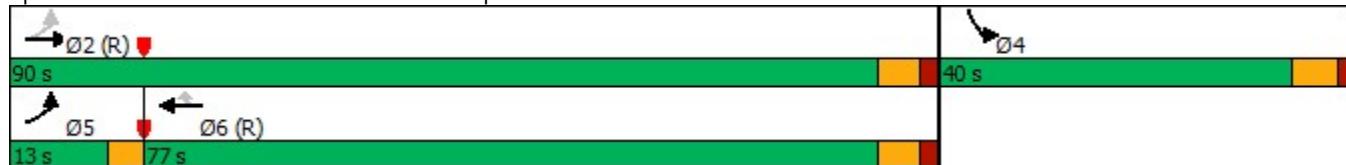
Intersection LOS: C

Intersection Capacity Utilization 73.5%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 1: Butterfield Road & E Loop Road



## Intersection

Int Delay, s/veh 9.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗											
Traffic Vol, veh/h	7	16	139	130	12	79	82	341	170	78	339	8
Future Vol, veh/h	7	16	139	130	12	79	82	341	170	78	339	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	0	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	0	0	1	0	0	0	0	0	0	0	1	0
Mvmt Flow	7	17	148	138	13	84	87	363	181	83	361	9

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	1208	1250	366	1242	1164	454	370	0	0	544	0	0
Stage 1	532	532	-	628	628	-	-	-	-	-	-	-
Stage 2	676	718	-	614	536	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.21	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.309	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	161	174	681	153	196	610	1200	-	-	1035	-	-
Stage 1	535	529	-	474	479	-	-	-	-	-	-	-
Stage 2	446	436	-	483	527	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	119	149	681	~101	167	610	1200	-	-	1035	-	-
Mov Cap-2 Maneuver	204	240	-	186	268	-	-	-	-	-	-	-
Stage 1	496	487	-	440	445	-	-	-	-	-	-	-
Stage 2	346	405	-	336	485	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	14.2	44.2			1.1			1.6		
HCM LOS	B	E								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1200	-	-	204	572	186	522	1035	-	-
HCM Lane V/C Ratio	0.073	-	-	0.037	0.288	0.744	0.185	0.08	-	-
HCM Control Delay (s)	8.2	-	-	23.3	13.8	65.7	13.5	8.8	-	-
HCM Lane LOS	A	-	-	C	B	F	B	A	-	-
HCM 95th %tile Q(veh)	0.2	-	-	0.1	1.2	4.8	0.7	0.3	-	-

## Notes

~: Volume exceeds capacity    \$: Delay exceeds 300s    +: Computation Not Defined    \*: All major volume in platoon

## Intersection

Int Delay, s/veh

2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖
Traffic Vol, veh/h	10	0	30	55	0	19	27	388	12	5	340	8
Future Vol, veh/h	10	0	30	55	0	19	27	388	12	5	340	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	0	-	-	-	150	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	98	98	98	98	98	98	98	98	98	98	98	98
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	10	0	31	56	0	19	28	396	12	5	347	8

Major/Minor	Minor2	Minor1			Major1			Major2			
Conflicting Flow All	829	-	351	835	823	402	355	0	0	408	0
Stage 1	361	-	-	458	458	-	-	-	-	-	-
Stage 2	468	-	-	377	365	-	-	-	-	-	-
Critical Hdwy	7.1	-	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-
Critical Hdwy Stg 1	6.1	-	-	6.1	5.5	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	-	-	6.1	5.5	-	-	-	-	-	-
Follow-up Hdwy	3.5	-	3.3	3.5	4	3.3	2.2	-	-	2.2	-
Pot Cap-1 Maneuver	292	0	697	289	311	653	1215	-	-	1162	-
Stage 1	662	0	-	587	570	-	-	-	-	-	-
Stage 2	579	0	-	649	627	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-
Mov Cap-1 Maneuver	277	-	697	271	303	653	1215	-	-	1162	-
Mov Cap-2 Maneuver	394	-	-	389	401	-	-	-	-	-	-
Stage 1	647	-	-	573	557	-	-	-	-	-	-
Stage 2	549	-	-	618	624	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	11.4	15	0.5	0.1
HCM LOS	B	C		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1215	-	-	394	697	434	1162	-	-
HCM Lane V/C Ratio	0.023	-	-	0.026	0.044	0.174	0.004	-	-
HCM Control Delay (s)	8	-	-	14.4	10.4	15	8.1	-	-
HCM Lane LOS	A	-	-	B	B	C	A	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.1	0.6	0	-	-

Intersection												
Int Delay, s/veh	3.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	47	308	57	1	338	78	67	18	20	25	8	59
Future Vol, veh/h	47	308	57	1	338	78	67	18	20	25	8	59
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	100	-	-	0	-	-	-	-	100
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	2
Mvmt Flow	55	358	66	1	393	91	78	21	23	29	9	69
Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	484	0	0	424	0	0	981	987	391	964	975	439
Stage 1	-	-	-	-	-	-	501	501	-	441	441	-
Stage 2	-	-	-	-	-	-	480	486	-	523	534	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.318
Pot Cap-1 Maneuver	1089	-	-	1146	-	-	231	249	662	237	253	618
Stage 1	-	-	-	-	-	-	556	546	-	599	580	-
Stage 2	-	-	-	-	-	-	571	554	-	541	528	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1089	-	-	1146	-	-	193	236	662	210	240	618
Mov Cap-2 Maneuver	-	-	-	-	-	-	307	339	-	331	353	-
Stage 1	-	-	-	-	-	-	528	518	-	568	579	-
Stage 2	-	-	-	-	-	-	499	553	-	476	501	-
Approach	EB	WB		NE		SW						
HCM Control Delay, s	1	0		18.2		13.6						
HCM LOS				C		B						
Minor Lane/Major Mvmt	NELn1	NELn2	EBL	EBT	EBR	WBL	WBT	WBR	SWLn1	SWLn2		
Capacity (veh/h)	307	456	1089	-	-	1146	-	-	336	618		
HCM Lane V/C Ratio	0.254	0.097	0.05	-	-	0.001	-	-	0.114	0.111		
HCM Control Delay (s)	20.7	13.7	8.5	-	-	8.1	-	-	17.1	11.6		
HCM Lane LOS	C	B	A	-	-	A	-	-	C	B		
HCM 95th %tile Q(veh)	1	0.3	0.2	-	-	0	-	-	0.4	0.4		

## Capacity Analysis Summary Sheets

Year 2028 Total Projected Weekday Evening Peak Hour  
Without Restaurant

Lanes, Volumes, Timings  
1: Butterfield Road & E Loop Road

02/13/2023



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑	↑↑	↑↑	↑	↑↑	
Traffic Volume (vph)	121	1263	1366	454	497	110
Future Volume (vph)	121	1263	1366	454	497	110
Ideal Flow (vphpl)	1900	2000	2000	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	230			220	225	150
Storage Lanes	1			1	1	0
Taper Length (ft)	250				55	
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Fr <sub>t</sub>				0.850	0.973	
Flt Protected	0.950				0.961	
Satd. Flow (prot)	1787	3725	3800	1615	3428	0
Flt Permitted	0.108				0.961	
Satd. Flow (perm)	203	3725	3800	1615	3428	0
Right Turn on Red				No	No	
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		30	
Link Distance (ft)		1427	1045		481	
Travel Time (s)		21.6	15.8		10.9	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	2%	0%	0%	0%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	123	1289	1394	463	619	0
Turn Type	pm+pt	NA	NA	Perm	Prot	
Protected Phases	5	2	6		4	
Permitted Phases	2			6		
Detector Phase	5	2	6	6	4	
Switch Phase						
Minimum Initial (s)	3.0	15.0	15.0	15.0	8.0	
Minimum Split (s)	12.5	24.5	24.5	24.5	24.0	
Total Split (s)	13.0	90.0	77.0	77.0	40.0	
Total Split (%)	10.0%	69.2%	59.2%	59.2%	30.8%	
Yellow Time (s)	3.5	4.0	4.0	4.0	4.5	
All-Red Time (s)	0.0	2.0	2.0	2.0	1.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Min	C-Min	C-Min	None	
Act Effct Green (s)	89.8	87.3	75.5	75.5	30.7	
Actuated g/C Ratio	0.69	0.67	0.58	0.58	0.24	



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.51	0.52	0.63	0.49	0.77	
Control Delay	14.8	12.0	20.3	19.1	52.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	14.8	12.0	20.3	19.1	52.9	
LOS	B	B	C	B	D	
Approach Delay		12.2	20.0		52.9	
Approach LOS		B	C		D	
Queue Length 50th (ft)	32	272	401	225	249	
Queue Length 95th (ft)	58	343	504	337	309	
Internal Link Dist (ft)		1347	965		401	
Turn Bay Length (ft)	230		220	225		
Base Capacity (vph)	255	2501	2208	938	896	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.48	0.52	0.63	0.49	0.69	

#### Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 130

Offset: 23 (18%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.77

Intersection Signal Delay: 22.4

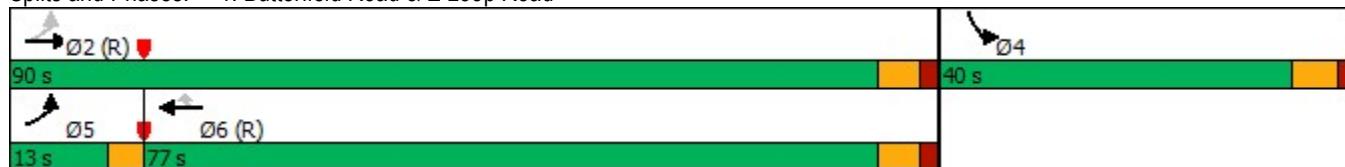
Intersection LOS: C

Intersection Capacity Utilization 73.5%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 1: Butterfield Road & E Loop Road



## Intersection

Int Delay, s/veh 7.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗ ↗											
Traffic Vol, veh/h	7	16	139	116	12	70	82	341	147	63	339	8
Future Vol, veh/h	7	16	139	116	12	70	82	341	147	63	339	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	-	0	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	0	0	1	0	0	0	0	0	0	0	1	0
Mvmt Flow	7	17	148	123	13	74	87	363	156	67	361	9

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	1159	1193	366	1197	1119	441	370	0	0	519	0	0
Stage 1	500	500	-	615	615	-	-	-	-	-	-	-
Stage 2	659	693	-	582	504	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.21	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.309	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	174	188	681	164	209	621	1200	-	-	1057	-	-
Stage 1	557	546	-	482	485	-	-	-	-	-	-	-
Stage 2	456	448	-	502	544	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	133	164	681	~ 110	182	621	1200	-	-	1057	-	-
Mov Cap-2 Maneuver	225	260	-	199	282	-	-	-	-	-	-	-
Stage 1	517	512	-	447	450	-	-	-	-	-	-	-
Stage 2	362	416	-	356	510	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	13.9	34.1	1.2	1.3
HCM LOS	B	D		
<hr/>				
Minor Lane/Major Mvmt	NBL	NBT	NBR	E BLn1 E BLn2 W BLn1 W BLn2
Capacity (veh/h)	1200	-	-	225 583 199 528
HCM Lane V/C Ratio	0.073	-	-	0.033 0.283 0.62 0.165
HCM Control Delay (s)	8.2	-	-	21.5 13.6 48.8 13.2
HCM Lane LOS	A	-	-	C B E B A
HCM 95th %tile Q(veh)	0.2	-	-	0.1 1.2 3.6 0.6

## Notes

~: Volume exceeds capacity    \$: Delay exceeds 300s    +: Computation Not Defined    \*: All major volume in platoon

## Intersection

Int Delay, s/veh 2.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖
Traffic Vol, veh/h	10	0	30	55	0	19	27	379	12	5	325	8
Future Vol, veh/h	10	0	30	55	0	19	27	379	12	5	325	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	0	-	-	-	150	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	98	98	98	98	98	98	98	98	98	98	98	98
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	10	0	31	56	0	19	28	387	12	5	332	8

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	805	-	336	811	799	393	340	0	0	399	0	0
Stage 1	346	-	-	449	449	-	-	-	-	-	-	-
Stage 2	459	-	-	362	350	-	-	-	-	-	-	-
Critical Hdwy	7.1	-	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	-	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	303	0	711	300	321	660	1230	-	-	1171	-	-
Stage 1	674	0	-	593	576	-	-	-	-	-	-	-
Stage 2	586	0	-	661	636	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	288	-	711	281	312	660	1230	-	-	1171	-	-
Mov Cap-2 Maneuver	403	-	-	398	408	-	-	-	-	-	-	-
Stage 1	658	-	-	579	563	-	-	-	-	-	-	-
Stage 2	556	-	-	630	633	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	11.3	14.8			0.5			0.1		
HCM LOS	B	B								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1230	-	-	403	711	443	1171	-	-	
HCM Lane V/C Ratio	0.022	-	-	0.025	0.043	0.17	0.004	-	-	
HCM Control Delay (s)	8	-	-	14.2	10.3	14.8	8.1	-	-	
HCM Lane LOS	A	-	-	B	B	B	A	-	-	
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.1	0.6	0	-	-	

Intersection												
Int Delay, s/veh	3.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	47	293	57	1	329	78	67	18	20	25	8	59
Future Vol, veh/h	47	293	57	1	329	78	67	18	20	25	8	59
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	100	-	-	0	-	-	-	-	100
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	2
Mvmt Flow	55	341	66	1	383	91	78	21	23	29	9	69
Major/Minor												
Major1		Major2			Minor1		Minor2					
Conflicting Flow All	474	0	0	407	0	0	954	960	374	937	948	429
Stage 1	-	-	-	-	-	-	484	484	-	431	431	-
Stage 2	-	-	-	-	-	-	470	476	-	506	517	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.318
Pot Cap-1 Maneuver	1099	-	-	1163	-	-	240	259	677	247	263	626
Stage 1	-	-	-	-	-	-	568	555	-	607	586	-
Stage 2	-	-	-	-	-	-	578	560	-	552	537	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1099	-	-	1163	-	-	201	246	677	219	250	626
Mov Cap-2 Maneuver	-	-	-	-	-	-	315	347	-	339	362	-
Stage 1	-	-	-	-	-	-	540	527	-	577	585	-
Stage 2	-	-	-	-	-	-	506	559	-	486	510	-
Approach												
EB			WB			NE			SW			
HCM Control Delay, s	1			0			17.7		13.4			
HCM LOS							C		B			
Minor Lane/Major Mvmt												
Capacity (veh/h)	315	467	1099	-	-	1163	-	-	344	626		
HCM Lane V/C Ratio	0.247	0.095	0.05	-	-	0.001	-	-	0.112	0.11		
HCM Control Delay (s)	20.1	13.5	8.4	-	-	8.1	-	-	16.8	11.5		
HCM Lane LOS	C	B	A	-	-	A	-	-	C	B		
HCM 95th %tile Q(veh)	1	0.3	0.2	-	-	0	-	-	0.4	0.4		

## Capacity Analysis Summary Sheets

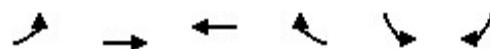
Year 2028 Total Projected Saturday Midday Peak Hour  
With Restaurant

Lanes, Volumes, Timings  
1: Butterfield Road & E Loop Road

02/13/2023



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑	↑↑	↑↑	↑	↑↑	
Traffic Volume (vph)	123	1015	1237	442	463	119
Future Volume (vph)	123	1015	1237	442	463	119
Ideal Flow (vphpl)	1900	2000	2000	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	230			220	225	150
Storage Lanes	1			1	1	0
Taper Length (ft)	250				55	
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Fr1				0.850	0.969	
Flt Protected	0.950				0.962	
Satd. Flow (prot)	1805	3762	3762	1615	3429	0
Flt Permitted	0.130				0.962	
Satd. Flow (perm)	247	3762	3762	1615	3429	0
Right Turn on Red				No	No	
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		30	
Link Distance (ft)		1427	1045		481	
Travel Time (s)		21.6	15.8		10.9	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	1%	1%	0%	0%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	127	1046	1275	456	600	0
Turn Type	pm+pt	NA	NA	Perm	Prot	
Protected Phases	5	2	6		4	
Permitted Phases	2			6		
Detector Phase	5	2	6	6	4	
Switch Phase						
Minimum Initial (s)	3.0	15.0	15.0	15.0	8.0	
Minimum Split (s)	12.5	24.5	24.5	24.5	24.0	
Total Split (s)	13.0	80.0	67.0	67.0	40.0	
Total Split (%)	10.8%	66.7%	55.8%	55.8%	33.3%	
Yellow Time (s)	3.5	4.0	4.0	4.0	4.5	
All-Red Time (s)	0.0	2.0	2.0	2.0	1.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Min	C-Min	C-Min	None	
Act Effct Green (s)	81.8	79.3	67.6	67.6	28.7	
Actuated g/C Ratio	0.68	0.66	0.56	0.56	0.24	



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.46	0.42	0.60	0.50	0.73	
Control Delay	12.9	10.7	19.7	19.5	47.4	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	12.9	10.7	19.7	19.5	47.4	
LOS	B	B	B	B	D	
Approach Delay		10.9	19.7		47.4	
Approach LOS		B	B		D	
Queue Length 50th (ft)	31	187	334	209	220	
Queue Length 95th (ft)	61	258	450	336	270	
Internal Link Dist (ft)		1347	965		401	
Turn Bay Length (ft)	230		220	225		
Base Capacity (vph)	291	2485	2118	909	971	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.44	0.42	0.60	0.50	0.62	

#### Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 29 (24%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.73

Intersection Signal Delay: 21.5

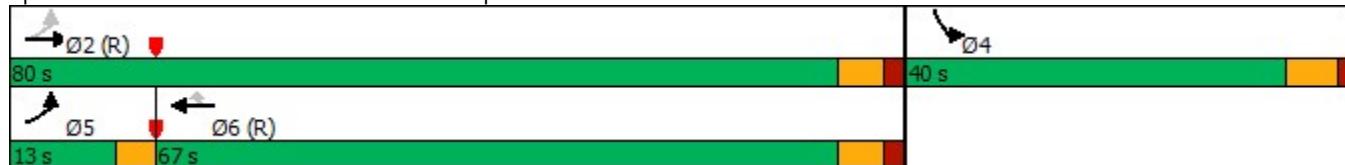
Intersection LOS: C

Intersection Capacity Utilization 69.6%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: Butterfield Road & E Loop Road



## Intersection

Int Delay, s/veh 13

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	13	16	182	151	13	87	100	268	164	56	243	16
Future Vol, veh/h	13	16	182	151	13	87	100	268	164	56	243	16
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	-	0	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	0	1	0	0	0	0	0	0	0	0
Mvmt Flow	14	18	200	166	14	96	110	295	180	62	267	18

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	1060	1095	276	1114	1014	385	285	0	0	475	0	0
Stage 1	400	400	-	605	605	-	-	-	-	-	-	-
Stage 2	660	695	-	509	409	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.11	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.11	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.11	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.509	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	204	215	768	186	240	667	1289	-	-	1098	-	-
Stage 1	630	605	-	486	491	-	-	-	-	-	-	-
Stage 2	455	447	-	549	600	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	151	186	768	~117	207	667	1289	-	-	1098	-	-
Mov Cap-2 Maneuver	230	273	-	198	301	-	-	-	-	-	-	-
Stage 1	576	571	-	445	449	-	-	-	-	-	-	-
Stage 2	345	409	-	371	566	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	13.4	51.5			1.5			1.5		
HCM LOS	B	F								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1289	-	-	230	670	198	576	1098	-	-
HCM Lane V/C Ratio	0.085	-	-	0.062	0.325	0.838	0.191	0.056	-	-
HCM Control Delay (s)	8.1	-	-	21.7	12.9	77.2	12.7	8.5	-	-
HCM Lane LOS	A	-	-	C	B	F	B	A	-	-
HCM 95th %tile Q(veh)	0.3	-	-	0.2	1.4	6.1	0.7	0.2	-	-

## Notes

~: Volume exceeds capacity    \$: Delay exceeds 300s    +: Computation Not Defined    \*: All major volume in platoon

## Intersection

Int Delay, s/veh 1.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖		↖		↖	↖	↖	↖	↖	↖	↖	↖
Traffic Vol, veh/h	9	0	31	18	0	6	27	323	18	6	266	8
Future Vol, veh/h	9	0	31	18	0	6	27	323	18	6	266	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	0	-	-	-	150	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	10	0	34	20	0	7	29	351	20	7	289	9

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	731	-	294	744	731	361	298	0	0	371	0	0
Stage 1	308	-	-	419	419	-	-	-	-	-	-	-
Stage 2	423	-	-	325	312	-	-	-	-	-	-	-
Critical Hdwy	7.1	-	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	-	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	340	0	750	333	351	688	1275	-	-	1199	-	-
Stage 1	706	0	-	616	593	-	-	-	-	-	-	-
Stage 2	613	0	-	692	661	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	329	-	750	311	341	688	1275	-	-	1199	-	-
Mov Cap-2 Maneuver	437	-	-	422	430	-	-	-	-	-	-	-
Stage 1	690	-	-	602	579	-	-	-	-	-	-	-
Stage 2	593	-	-	657	657	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	10.8	13.2			0.6			0.2		
HCM LOS	B	B								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1275	-	-	437	750	467	1199	-	-	
HCM Lane V/C Ratio	0.023	-	-	0.022	0.045	0.056	0.005	-	-	
HCM Control Delay (s)	7.9	-	-	13.4	10	13.2	8	-	-	
HCM Lane LOS	A	-	-	B	B	B	A	-	-	
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.1	0.2	0	-	-	

Intersection												
Int Delay, s/veh	3.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	43	246	72	0	308	30	83	22	2	32	15	39
Future Vol, veh/h	43	246	72	0	308	30	83	22	2	32	15	39
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	100	-	-	0	-	-	-	-	100
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	98	98	98	98	98	98	98	98	98	98	98	98
Heavy Vehicles, %	0	1	0	0	1	0	0	5	0	3	0	0
Mvmt Flow	44	251	73	0	314	31	85	22	2	33	15	40
Major/Minor												
Major1		Major2			Minor1		Minor2					
Conflicting Flow All	345	0	0	324	0	0	733	721	288	718	742	330
Stage 1	-	-	-	-	-	-	376	376	-	330	330	-
Stage 2	-	-	-	-	-	-	357	345	-	388	412	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.55	6.2	7.13	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.55	-	6.13	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.55	-	6.13	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4.045	3.3	3.527	4	3.3
Pot Cap-1 Maneuver	1225	-	-	1247	-	-	339	350	756	343	346	716
Stage 1	-	-	-	-	-	-	649	611	-	681	649	-
Stage 2	-	-	-	-	-	-	665	631	-	634	598	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1225	-	-	1247	-	-	303	337	756	319	334	716
Mov Cap-2 Maneuver	-	-	-	-	-	-	410	422	-	427	430	-
Stage 1	-	-	-	-	-	-	626	589	-	656	649	-
Stage 2	-	-	-	-	-	-	613	631	-	586	576	-
Approach												
EB			WB			NE			SW			
HCM Control Delay, s	1			0			15.6			12.6		
HCM LOS							C			B		
Minor Lane/Major Mvmt												
Capacity (veh/h)	410	438	1225	EBL	EBT	EBR	WBL	WBT	WBR	SWL	SWL	SWR
HCM Lane V/C Ratio	0.207	0.056	0.036	-	-	-	-	-	-	0.112	0.056	
HCM Control Delay (s)	16.1	13.7	8	-	-	-	0	-	-	14.5	10.3	
HCM Lane LOS	C	B	A	-	-	-	A	-	-	B	B	
HCM 95th %tile Q(veh)	0.8	0.2	0.1	-	-	-	0	-	-	0.4	0.2	

## Capacity Analysis Summary Sheets

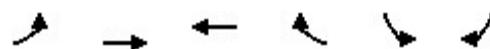
Year 2028 Total Projected Saturday Midday Peak Hour  
Without Restaurant

Lanes, Volumes, Timings  
1: Butterfield Road & E Loop Road

02/13/2023



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	↑	↑↑	↑↑	↑	↑↑	
Traffic Volume (vph)	115	1015	1237	426	459	111
Future Volume (vph)	115	1015	1237	426	459	111
Ideal Flow (vphpl)	1900	2000	2000	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)		0%	0%		0%	
Storage Length (ft)	230			220	225	150
Storage Lanes	1			1	1	0
Taper Length (ft)	250				55	
Lane Util. Factor	1.00	0.95	0.95	1.00	0.97	0.95
Ped Bike Factor						
Fr1				0.850	0.971	
Flt Protected	0.950				0.961	
Satd. Flow (prot)	1805	3762	3762	1615	3433	0
Flt Permitted	0.131				0.961	
Satd. Flow (perm)	249	3762	3762	1615	3433	0
Right Turn on Red				No	No	
Satd. Flow (RTOR)						
Link Speed (mph)		45	45		30	
Link Distance (ft)		1427	1045		481	
Travel Time (s)		21.6	15.8		10.9	
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	1%	1%	0%	0%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)		0%	0%		0%	
Shared Lane Traffic (%)						
Lane Group Flow (vph)	119	1046	1275	439	587	0
Turn Type	pm+pt	NA	NA	Perm	Prot	
Protected Phases	5	2	6		4	
Permitted Phases	2			6		
Detector Phase	5	2	6	6	4	
Switch Phase						
Minimum Initial (s)	3.0	15.0	15.0	15.0	8.0	
Minimum Split (s)	12.5	24.5	24.5	24.5	24.0	
Total Split (s)	13.0	80.0	67.0	67.0	40.0	
Total Split (%)	10.8%	66.7%	55.8%	55.8%	33.3%	
Yellow Time (s)	3.5	4.0	4.0	4.0	4.5	
All-Red Time (s)	0.0	2.0	2.0	2.0	1.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	3.5	6.0	6.0	6.0	6.0	
Lead/Lag	Lead		Lag	Lag		
Lead-Lag Optimize?	Yes		Yes	Yes		
Recall Mode	None	C-Min	C-Min	C-Min	None	
Act Effct Green (s)	82.1	79.6	68.1	68.1	28.4	
Actuated g/C Ratio	0.68	0.66	0.57	0.57	0.24	



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
v/c Ratio	0.43	0.42	0.60	0.48	0.72	
Control Delay	12.2	10.5	19.4	18.8	47.3	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	12.2	10.5	19.4	18.8	47.3	
LOS	B	B	B	B	D	
Approach Delay		10.7	19.2		47.3	
Approach LOS		B	B		D	
Queue Length 50th (ft)	29	185	329	196	215	
Queue Length 95th (ft)	57	258	450	320	263	
Internal Link Dist (ft)		1347	965		401	
Turn Bay Length (ft)	230			220	225	
Base Capacity (vph)	293	2496	2134	916	972	
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.41	0.42	0.60	0.48	0.60	

#### Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 29 (24%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.72

Intersection Signal Delay: 21.1

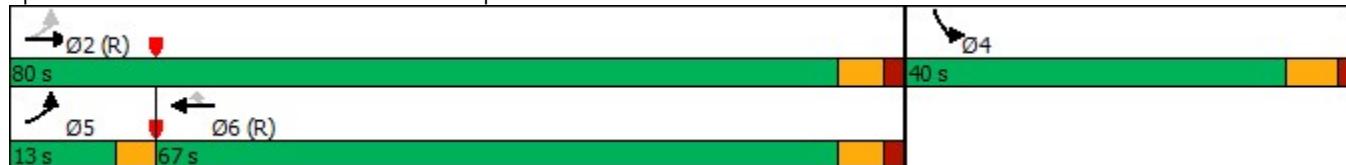
Intersection LOS: C

Intersection Capacity Utilization 68.8%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: Butterfield Road & E Loop Road



## Intersection

Int Delay, s/veh 9.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	13	16	182	128	13	72	100	268	140	40	243	16
Future Vol, veh/h	13	16	182	128	13	72	100	268	140	40	243	16
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	-	0	-	-	100	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	0	1	0	0	0	0	0	0	0	0
Mvmt Flow	14	18	200	141	14	79	110	295	154	44	267	18

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	1003	1033	276	1065	965	372	285	0	0	449	0	0
Stage 1	364	364	-	592	592	-	-	-	-	-	-	-
Stage 2	639	669	-	473	373	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.11	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.11	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.11	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.509	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	223	234	768	201	257	678	1289	-	-	1122	-	-
Stage 1	659	627	-	494	497	-	-	-	-	-	-	-
Stage 2	468	459	-	574	622	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	173	206	768	~129	226	678	1289	-	-	1122	-	-
Mov Cap-2 Maneuver	259	297	-	214	318	-	-	-	-	-	-	-
Stage 1	603	603	-	452	455	-	-	-	-	-	-	-
Stage 2	366	420	-	396	598	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	13.1	34.5			1.6			1.1		
HCM LOS	B	D								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1289	-	-	259	681	214	578	1122	-	-
HCM Lane V/C Ratio	0.085	-	-	0.055	0.32	0.657	0.162	0.039	-	-
HCM Control Delay (s)	8.1	-	-	19.7	12.7	49.2	12.4	8.3	-	-
HCM Lane LOS	A	-	-	C	B	E	B	A	-	-
HCM 95th %tile Q(veh)	0.3	-	-	0.2	1.4	4	0.6	0.1	-	-

## Notes

~: Volume exceeds capacity    \$: Delay exceeds 300s    +: Computation Not Defined    \*: All major volume in platoon

## Intersection

Int Delay, s/veh 1.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖		↖	↖	↖	↖	↖	↖	↖	↖	↖	↖
Traffic Vol, veh/h	9	0	31	18	0	6	27	308	18	6	250	8
Future Vol, veh/h	9	0	31	18	0	6	27	308	18	6	250	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	0	-	0	-	-	-	150	-	-	100	-	-
Veh in Median Storage, #	-	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	10	0	34	20	0	7	29	335	20	7	272	9

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	698	-	277	711	698	345	281	0	0	355	0	0
Stage 1	291	-	-	403	403	-	-	-	-	-	-	-
Stage 2	407	-	-	308	295	-	-	-	-	-	-	-
Critical Hdwy	7.1	-	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	-	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	-	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	358	0	767	351	367	702	1293	-	-	1215	-	-
Stage 1	721	0	-	628	603	-	-	-	-	-	-	-
Stage 2	625	0	-	706	673	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	347	-	767	329	357	702	1293	-	-	1215	-	-
Mov Cap-2 Maneuver	451	-	-	436	443	-	-	-	-	-	-	-
Stage 1	705	-	-	614	590	-	-	-	-	-	-	-
Stage 2	605	-	-	671	669	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	10.6	12.9			0.6			0.2		
HCM LOS	B	B								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR	
Capacity (veh/h)	1293	-	-	451	767	482	1215	-	-	
HCM Lane V/C Ratio	0.023	-	-	0.022	0.044	0.054	0.005	-	-	
HCM Control Delay (s)	7.8	-	-	13.2	9.9	12.9	8	-	-	
HCM Lane LOS	A	-	-	B	A	B	A	-	-	
HCM 95th %tile Q(veh)	0.1	-	-	0.1	0.1	0.2	0	-	-	

Intersection												
Int Delay, s/veh	3.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NEL	NET	NER	SWL	SWT	SWR
Lane Configurations	↖	↗		↖	↗		↖	↗		↖	↗	
Traffic Vol, veh/h	43	230	72	0	293	30	83	22	2	32	15	39
Future Vol, veh/h	43	230	72	0	293	30	83	22	2	32	15	39
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	100	-	-	100	-	-	0	-	-	-	-	100
Veh in Median Storage, #	-	0	-	-	0	-	-	1	-	-	1	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	98	98	98	98	98	98	98	98	98	98	98	98
Heavy Vehicles, %	0	1	0	0	1	0	0	5	0	3	0	0
Mvmt Flow	44	235	73	0	299	31	85	22	2	33	15	40
Major/Minor												
Major1		Major2			Minor1		Minor2					
Conflicting Flow All	330	0	0	308	0	0	702	690	272	687	711	315
Stage 1	-	-	-	-	-	-	360	360	-	315	315	-
Stage 2	-	-	-	-	-	-	342	330	-	372	396	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.55	6.2	7.13	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.55	-	6.13	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.55	-	6.13	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4.045	3.3	3.527	4	3.3
Pot Cap-1 Maneuver	1241	-	-	1264	-	-	355	364	772	360	361	730
Stage 1	-	-	-	-	-	-	662	621	-	694	659	-
Stage 2	-	-	-	-	-	-	677	640	-	646	607	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1241	-	-	1264	-	-	318	351	772	336	348	730
Mov Cap-2 Maneuver	-	-	-	-	-	-	422	433	-	441	441	-
Stage 1	-	-	-	-	-	-	639	599	-	670	659	-
Stage 2	-	-	-	-	-	-	625	640	-	598	586	-
Approach												
EB			WB			NE			SW			
HCM Control Delay, s	1			0			15.2			12.4		
HCM LOS							C			B		
Minor Lane/Major Mvmt												
Capacity (veh/h)	422	449	1241	-	-	1264	-	-	441	730		
HCM Lane V/C Ratio	0.201	0.055	0.035	-	-	-	-	-	0.109	0.055		
HCM Control Delay (s)	15.7	13.5	8	-	-	0	-	-	14.2	10.2		
HCM Lane LOS	C	B	A	-	-	A	-	-	B	B		
HCM 95th %tile Q(veh)	0.7	0.2	0.1	-	-	0	-	-	0.4	0.2		